

Burton town line to  
Burton Village

79

DIETZGEN  
 TRADE MARK

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ENGINEERS'  
FIELD BOOK

No. 400

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# EUGENE DIETZGEN CO.

DRAWING MATERIALS, MATHEMATICAL and  
SURVEYING INSTRUMENTS

Chicago New York San Francisco New Orleans Pittsburg Toronto

Distances from center of roadway for cross-sectioning  
Roadway 10 feet wide Side slopes 1 on 1  
For Single Track Embankment

PLEASE RETURN TO  
GEAUGA COUNTY ENGINEER

H	0	.1	.2	.3	.4	.5	.6	.7	.8	.9	H
0	8.0	8.1	8.2	8.3	8.4	8.5	8.6	8.7	8.8	8.9	0
1	9.0	9.1	9.2	9.3	9.4	9.5	9.6	9.7	9.8	9.9	1
2	10.0	10.1	10.2	10.3	10.4	10.5	10.6	10.7	10.8	10.9	2
3	11.0	11.1	11.2	11.3	11.4	11.5	11.6	11.7	11.8	11.9	3
4	12.0	12.1	12.2	12.3	12.4	12.5	12.6	12.7	12.8	12.9	4
5	13.0	13.1	13.2	13.3	13.4	13.5	13.6	13.7	13.8	13.9	5
6	14.0	14.1	14.2	14.3	14.4	14.5	14.6	14.7	14.8	14.9	6
7	15.0	15.1	15.2	15.3	15.4	15.5	15.6	15.7	15.8	15.9	7
8	16.0	16.1	16.2	16.3	16.4	16.5	16.6	16.7	16.8	16.9	8
9	17.0	17.1	17.2	17.3	17.4	17.5	17.6	17.7	17.8	17.9	9
10	18.0	18.1	18.2	18.3	18.4	18.5	18.6	18.7	18.8	18.9	10
11	19.0	19.1	19.2	19.3	19.4	19.5	19.6	19.7	19.8	19.9	11
12	20.0	20.1	20.2	20.3	20.4	20.5	20.6	20.7	20.8	20.9	12
13	21.0	21.1	21.2	21.3	21.4	21.5	21.6	21.7	21.8	21.9	13
14	22.0	22.1	22.2	22.3	22.4	22.5	22.6	22.7	22.8	22.9	14
15	23.0	23.1	23.2	23.3	23.4	23.5	23.6	23.7	23.8	23.9	15
16	24.0	24.1	24.2	24.3	24.4	24.5	24.6	24.7	24.8	24.9	16
17	25.0	25.1	25.2	25.3	25.4	25.5	25.6	25.7	25.8	25.9	17
18	26.0	26.1	26.2	26.3	26.4	26.5	26.6	26.7	26.8	26.9	18
19	27.0	27.1	27.2	27.3	27.4	27.5	27.6	27.7	27.8	27.9	19
20	28.0	28.1	28.2	28.3	28.4	28.5	28.6	28.7	28.8	28.9	20
21	29.0	29.1	29.2	29.3	29.4	29.5	29.6	29.7	29.8	29.9	21
22	30.0	30.1	30.2	30.3	30.4	30.5	30.6	30.7	30.8	30.9	22
23	31.0	31.1	31.2	31.3	31.4	31.5	31.6	31.7	31.8	31.9	23
24	32.0	32.1	32.2	32.3	32.4	32.5	32.6	32.7	32.8	32.9	24
25	33.0	33.1	33.2	33.3	33.4	33.5	33.6	33.7	33.8	33.9	25
26	34.0	34.1	34.2	34.3	34.4	34.5	34.6	34.7	34.8	34.9	26
27	35.0	35.1	35.2	35.3	35.4	35.5	35.6	35.7	35.8	35.9	27
28	36.0	36.1	36.2	36.3	36.4	36.5	36.6	36.7	36.8	36.9	28
29	37.0	37.1	37.2	37.3	37.4	37.5	37.6	37.7	37.8	37.9	29
30	38.0	38.1	38.2	38.3	38.4	38.5	38.6	38.7	38.8	38.9	30
31	39.0	39.1	39.2	39.3	39.4	39.5	39.6	39.7	39.8	39.9	31
32	40.0	40.1	40.2	40.3	40.4	40.5	40.6	40.7	40.8	40.9	32
33	41.0	41.1	41.2	41.3	41.4	41.5	41.6	41.7	41.8	41.9	33
34	42.0	42.1	42.2	42.3	42.4	42.5	42.6	42.7	42.8	42.9	34
35	43.0	43.1	43.2	43.3	43.4	43.5	43.6	43.7	43.8	43.9	35
36	44.0	44.1	44.2	44.3	44.4	44.5	44.6	44.7	44.8	44.9	36
37	45.0	45.1	45.2	45.3	45.4	45.5	45.6	45.7	45.8	45.9	37
38	46.0	46.1	46.2	46.3	46.4	46.5	46.6	46.7	46.8	46.9	38
39	47.0	47.1	47.2	47.3	47.4	47.5	47.6	47.7	47.8	47.9	39
40	48.0	48.1	48.2	48.3	48.4	48.5	48.6	48.7	48.8	48.9	40

**Example**—If point is 22.6 ft. above grade, how far should it be from center line to be a slope stake point? Ans. from Table 30.6. For same slopes but other widths of roadbed, correct above figures by one-half difference in width of roadbed; thus in example above, for 20 ft. roadbed distance will be  $30.6 + (20 - 16) \div 2$  or 2 ft. added to  $30.6 = 32.6$ . For slopes of 1 on 1½ see inside of back cover.

Copyright, 1914, by Eugene Dietzgen Co.

Burton Town Line to Burton Vn.  
Hiram Rapido Rd #1 Sec E, F, G, H, OI Pg. 2

SW Burton Rd (CX No 273)

Georgia Rd

TR 124 GEORGIA RD (BURTON) Pg 28 E48

DATA ON Bridge AT W. Branch CUYAHOGA & CX #1 Pg. 35

White Road Random Line Pg 46

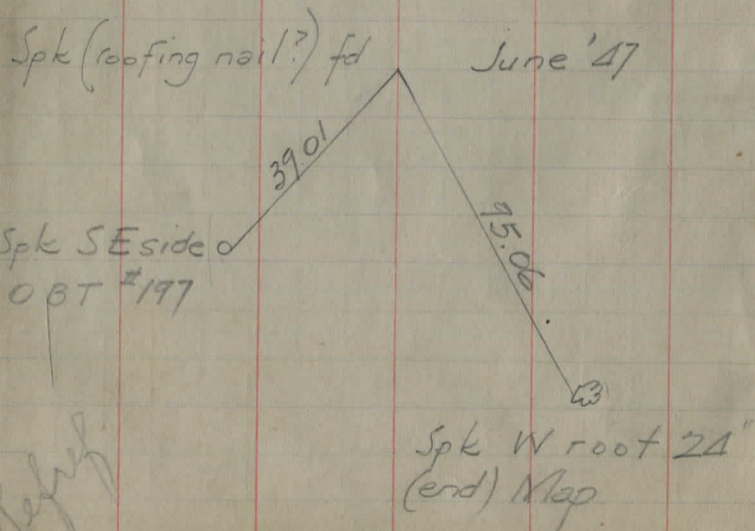
Chipmunk Lane Pg 50

TR 124 GEORGIA RD (Midd.) Pg 52/A80

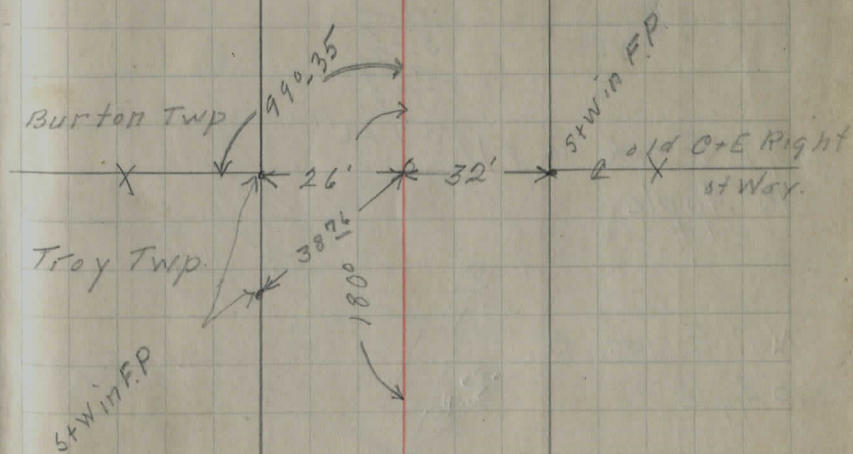
79

All stakes set 25' RT

# HIRAM RAPIDS RD.



Refresh



All stakes set rt.

± 12' 6-47

Sta 4 = .03  
 Sta 5 = 4.2  
 Sta 5+569 = 11.02  
 " 6 = 4.2  
 " 7 = 0.3

$$\begin{array}{r} 1.25 \\ 25 \\ \hline 125 \\ 50 \\ \hline .0425 \\ 8 \overline{) 4375} \end{array}$$

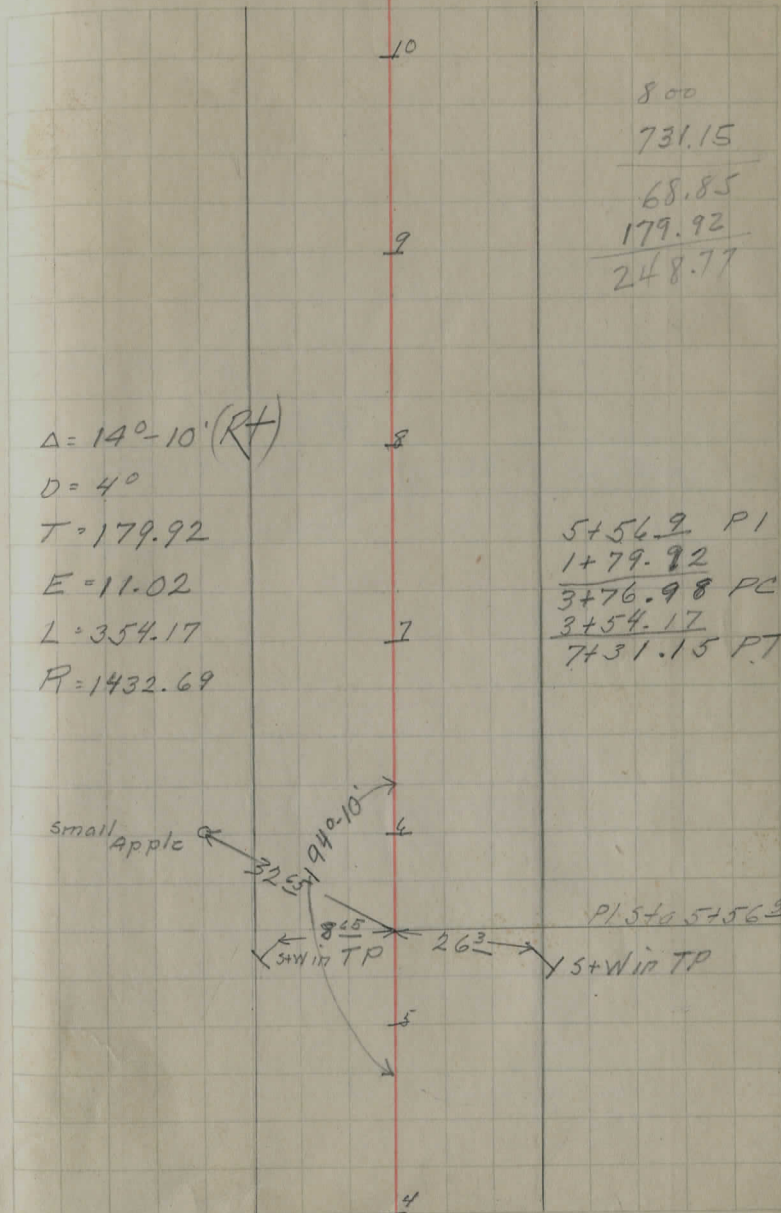
$$\begin{array}{r} 1.25 \\ 1.25 \\ \hline 6.25 \\ 250 \\ 125 \\ \hline 1.5425 \\ 7 \\ \hline 110.9375 \\ 4 \\ \hline 8 \overline{) 437500} \end{array}$$

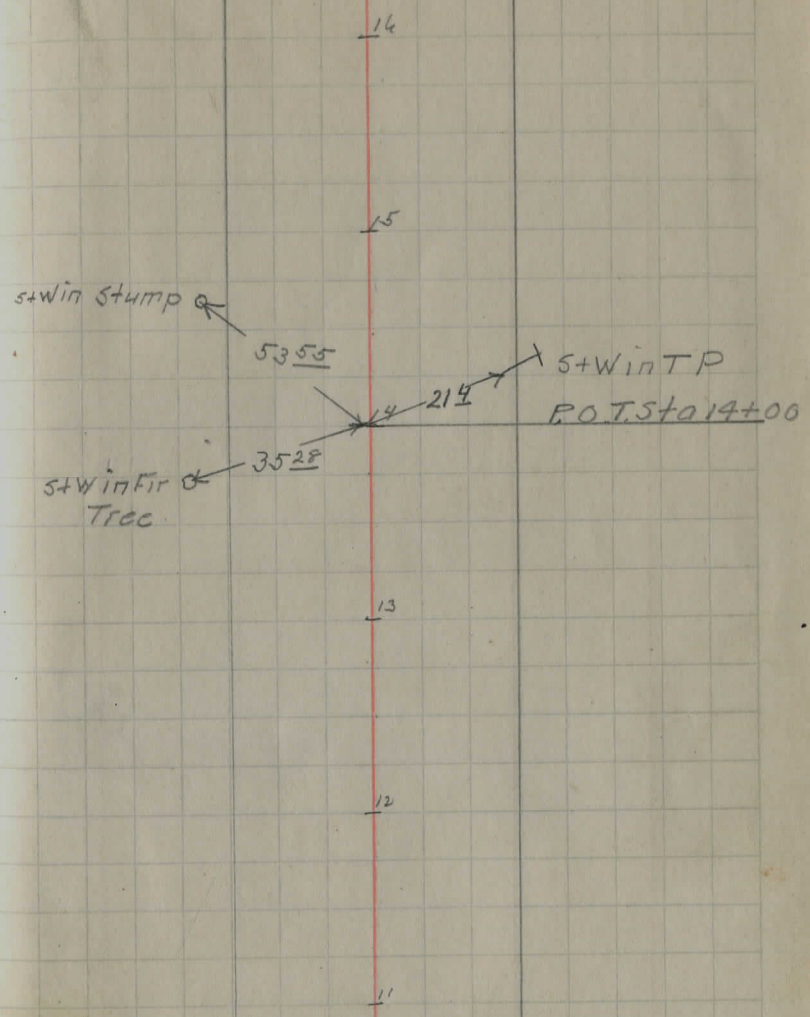
$$\begin{array}{r} 1.8 \\ 1.8 \\ \hline 144 \\ 18 \\ \hline 3.24 \\ 7 \\ \hline 2268 \\ 4 \\ \hline 56721 \end{array}$$

Spk E side @ 8) 90721"  
 10" Apple

Nail E side @ 32.85  
 10" Apple

Spk fd & replaced with I.P.  
 (in W. berm)





22

21

20

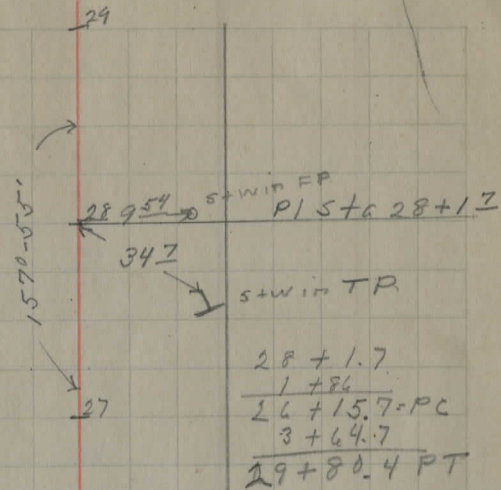
19

18

17

Offset on 100 cord = 5'

$$\begin{aligned}\Delta &= 220 - 0.5 \text{ Lt.} \\ D &= 60 \\ E &= 18.81 \\ T &= 186' \\ L &= 364.7 \\ R &= 955.37\end{aligned}$$



$$\begin{array}{r} 28 + 1.7 \\ 1 + 86 \\ \hline 26 + 15.7 = PC \\ 3 + 64.7 \\ \hline 29 + 80.4 PT \end{array}$$

$$\begin{array}{r} 3200 \\ 2980.4 \\ \hline 219.6 \\ 186 \\ \hline 405.6 \end{array}$$

35343332313029

41

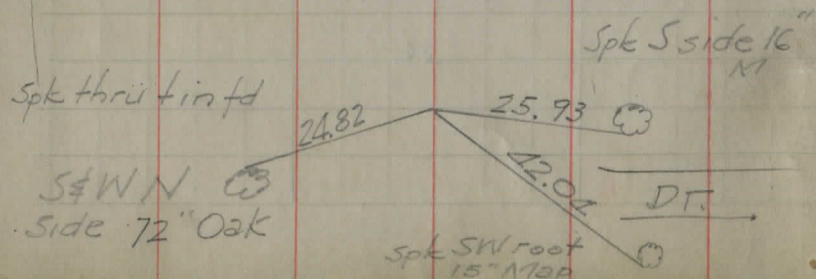
40

39

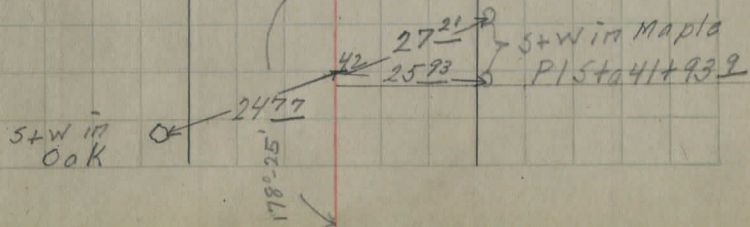
38

37

36



$$A = 1-35/t$$



54

53

52

51

↗

st Win Fir

58.95

st Win TP

16.8

50

Pista 50+0.00

33.92

st Win Maple

( $\Delta = 0^{\circ} 05' RT$ )

50-05 RT

49

48

62

39

53

58

61

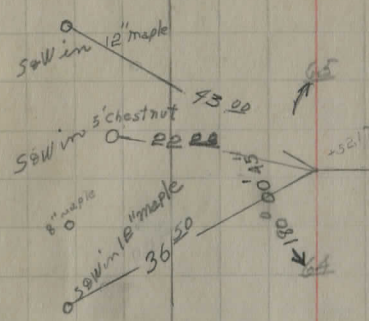
57

56

55

54

54



66

63

62

61

70

71

72

69

68

67

$$11) 132.5(11.1$$

$$\begin{array}{r} 11) 1239.7 \\ \underline{11} \\ 129 \\ \underline{11} \\ 189 \\ \underline{17} \\ 179 \\ \underline{17} \\ 97 \end{array}$$

$$\Delta = 240 - 25' \text{ Rt}$$

$$E = 11.1$$

$$T = 112.7$$

$$D = 110$$

$$L = 221.97$$

$$Eq4 = 3.4$$

$$\begin{array}{r} 112.7 \\ \underline{225.4} \\ 222.0 \\ \hline 3.4 \text{ Eq} \end{array}$$

77

$$133.0$$

$$11) 24416.7(221.97$$

$$\begin{array}{r} 22 \\ \underline{24} \\ 22 \\ \underline{21} \\ 106 \\ \underline{99} \\ 77 \end{array}$$

$$\begin{array}{r} 78 + 85.5 \\ \underline{1 + 22.7} \\ 77 + 72.8 \text{ PC} \\ \underline{2 + 21.97} \\ 79 + 94.77 \text{ PT} \end{array}$$

8000

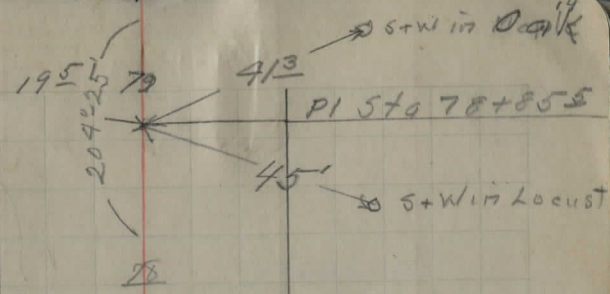
7994.77

05.23

112.7

117.93

St Win &  
Oaknut



78

77

76

75

74

73

June 47  
H<sub>2</sub>O oozing out

$$\begin{array}{r} 78 + 70 \\ \underline{+ 40' \text{ S of}} \end{array}$$

48" oak  
4' W of E  
edge part

Field Book 29

Rapids Road  
at Longhoga River

106+31.8  
 $\Delta=25-30R$   
Pg 19

842'

97+89.1  
Pg. 17 (top)  
 $\Delta=33-50R$

SPK  
Set  
5-3-57  
N.H.

489.1

93+00  
 $\Delta=8-40L$   
Pg. 17 bottom

SPK

See  
5-3-57  
N.H.

46.8

188'

BM = 90.75  
Pg. 39

$\Delta=24-27L$

80.0

I.P.  
Basat  
5-3-57  
N.H.

91+12

See pg 16

1226.5  
960'  
#160

SOW (d. 5-3-58 N.H.)  
Pg. 15 (bottom)

top Pg 14  
 $\Delta=24-25R$

85

June 17

84

84+20 Spring  
in West berm

83

82

81

80

79

fd to ba 960.0' m 5-3-58

SEN in  
Tel pole  
S side  
E -95

23 1/2

SW set (split part)

7-30-51 fd 5-3-58

2nd pole N of 40" Oak E side of

curve ± 79+0

$$\Delta = 24^{\circ} 27' \text{ Lt } R = 358.10$$

$$D = 16'$$

$$E = 8.4$$

$$T = 77.7$$

$$L = 153.2$$

$$Eq4 = 2.2$$

$$\begin{array}{r} 81 \overline{) 1244 \ 14} \\ \underline{81} \\ 434 \end{array}$$

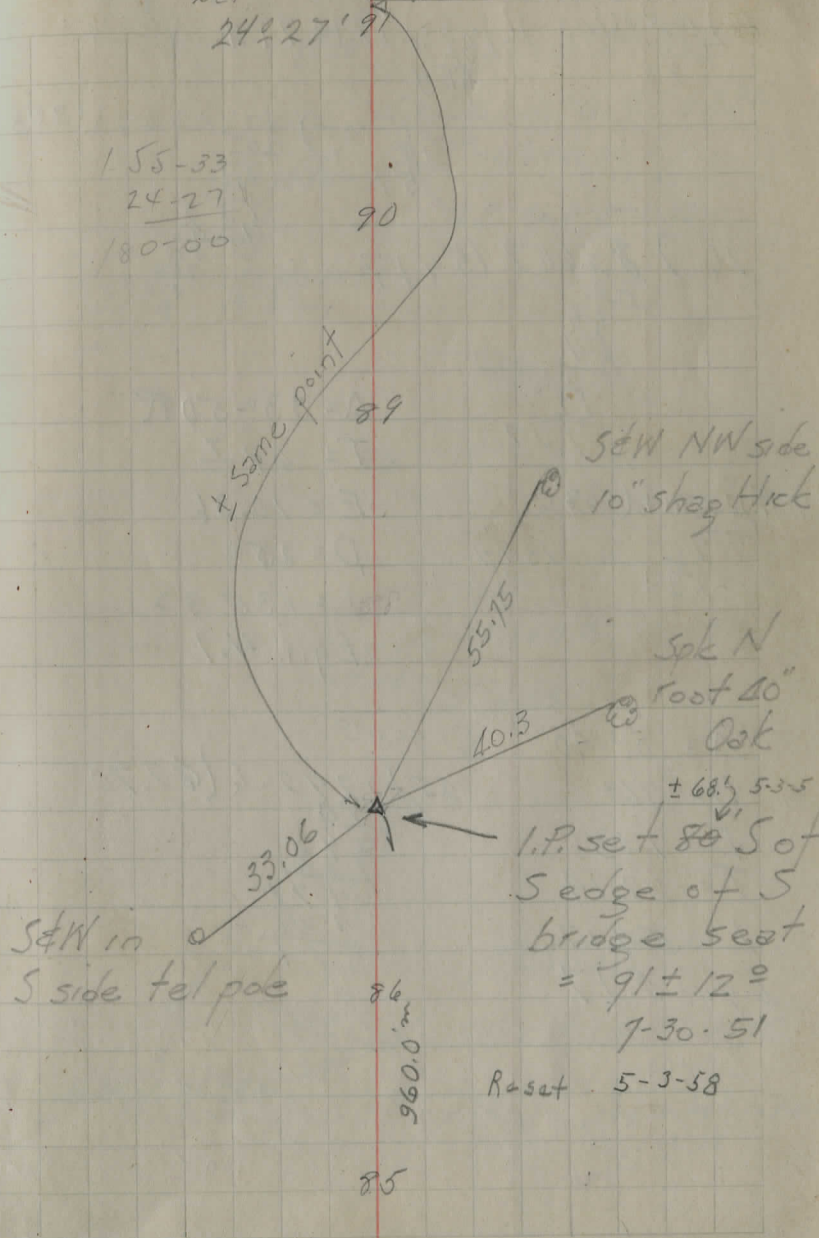
$$\begin{array}{r} 16 \overline{) 1244.00 \ 177.7} \\ \underline{112} \\ 124 \quad \underline{155.4} \\ \underline{112} \\ 120 \quad \underline{53.2} \\ \underline{120} \\ 2.2 \end{array}$$

$$\begin{array}{r} 16 \overline{) 133.5 \ 8.4} \\ \underline{128} \\ 5.5 \end{array}$$

$$\begin{array}{r} 16 \overline{) 24.50 \ 153.2} \\ \underline{16} \\ 85 \\ \underline{80} \\ 50 \\ \underline{48} \end{array}$$

$$\begin{array}{r} 91 + 12 \\ \underline{77.7} \\ 90 + 34.3 \\ \underline{153.2} \\ 91487.5 \end{array}$$

StW in Elm  $\rightarrow$  312  $\rightarrow$  40  $\rightarrow$  StW in oak  
Det  $\rightarrow$  P157091+12  $\rightarrow$  16



46) 16,43003 16) 434.17 | 27.14

$$\begin{array}{r} 16 \overline{) 1643003} \\ \underline{16} \\ 43 \\ \underline{43} \\ 00 \\ \underline{00} \\ 03 \end{array}$$

$$\begin{array}{r} 16 \overline{) 434.17} \\ \underline{32} \\ 114 \\ \underline{112} \\ 21 \\ \underline{16} \\ 57 \end{array}$$

$$25 \overline{) 33.8333} \quad (13532)$$

$$\begin{array}{r} 25 \overline{) 33.8333} \\ \underline{25} \\ 88 \\ \underline{75} \\ 133 \\ \underline{125} \\ 80 \\ \underline{75} \\ 50 \end{array}$$

16) 8.6667 | 54.17

$$\begin{array}{r} 16 \overline{) 8.6667} \\ \underline{16} \\ 26 \\ \underline{16} \\ 107 \end{array}$$

$\Delta = 330.50R$

$T = 49Z$

$E = 10.36$

$D = 25^\circ$

$L = 135.32$

Equ. 4.1

25) 2.5914 | 103.6

$$\begin{array}{r} 25 \overline{) 2.5914} \\ \underline{75} \\ 91 \\ \underline{75} \\ 160 \end{array}$$

1394  
1353

$\Delta = 80.40$

$D = 16^\circ$

$E = 1.03$

$T = 27.14$

$L = 54.17$

Equ: 0.17

$E = 358.10$

25) 1742.6 | 69.70

$$\begin{array}{r} 25 \overline{) 1742.6} \\ \underline{150} \\ 242 \\ \underline{225} \\ 171 \\ \underline{175} \\ 10 \end{array}$$

9255.7

922

63.5

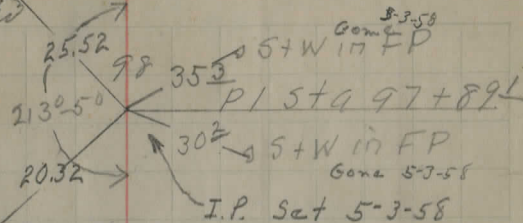
102

9300

27.14

72.86

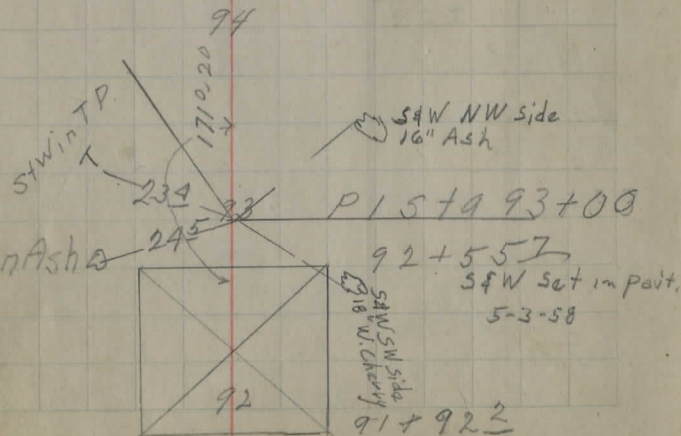
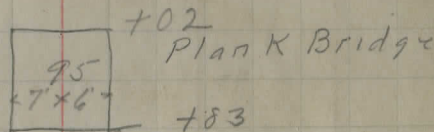
SPK Nside  
20" Twin Hickory



100.00  
9789.1  
210.9  
4.1  
215

SPK Nside 97  
14" Elm

96



+65 Old Wood Bridge  
+45 No good.

104

103

102

101

100

99

+61 Wood Floor  
Steel Bridge  
+42

7'x7'

$16 \overline{) 25.5} \quad 1159.38$   
 $\begin{array}{r} 16 \\ 95 \\ \hline 80 \\ 150 \\ \hline 144 \\ 60 \\ \hline 48 \\ 7130 \end{array}$   
 $16 \overline{) 1296.5} \quad 81.03$   
 $\begin{array}{r} 16 \\ 128 \\ \hline 16 \\ 165 \end{array}$

$\Delta = 25^\circ - 30'$   
 $D = 16'$   
 $E = 9.05$   
 $T = 81.03$   
 $L = 159.38$   
 $E_{94} \quad 2.68$

$16 \overline{) 104.85} \quad 6.55$   
 $\begin{array}{r} 16 \\ 96 \\ \hline 85 \end{array}$

110

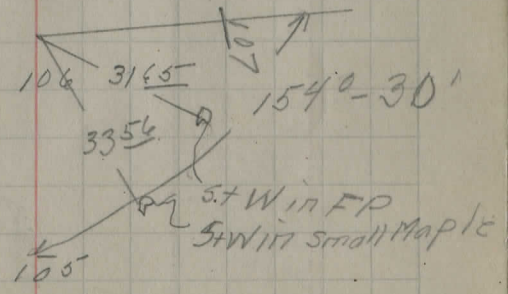
109

108

107

$10800$   
 $10631.8$   
 $\hline 168.2$   
 $2.68$   
 $\hline 170.9$   
 $31.8$   
 $106$   
 $9732$

Pista 106 + 31.8  
 Use 75' Rad  
 and Tangents on  
 outside



$\Delta 49^\circ - 45'$  Use 100' Radius  
T = in going around  
E = 10.54 Curve and  
D = 58' Width 2'  
L =  
EQU =

35<sup>55</sup> → SW in FP  
PI 5 + 114 + 890  
24<sup>15</sup> → SW in Mail Box

130°-15'

116

116

114

111

113

112

111

123

122

121

120

119

118

117

131

130

129

128

127

126

125

124

139

138

137

136

135

134

133

132



150 + 80  
620 scaled  

---

144 + 60 = + 5 Corps  
line  
Burton Dill

155

154

153

152

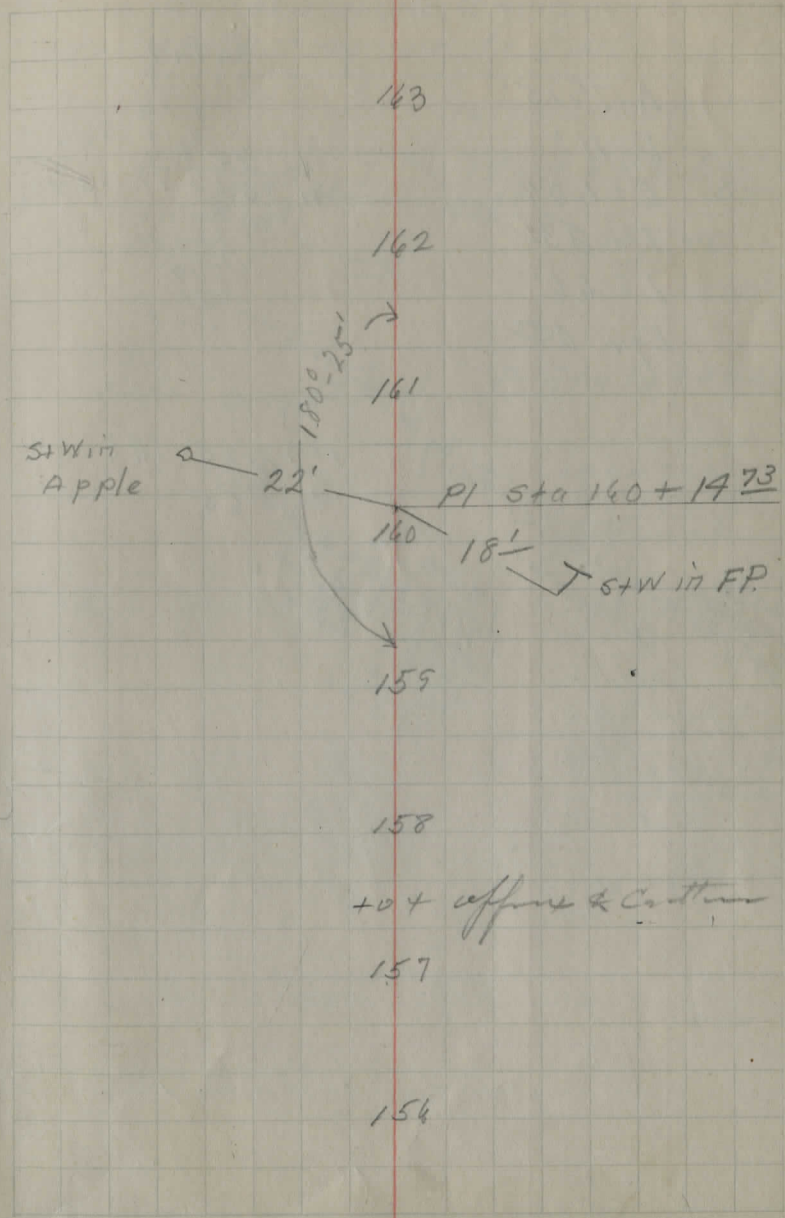
151

+80 of 1/2004. 2 Hotel Kissin

150

149

146

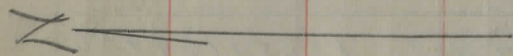




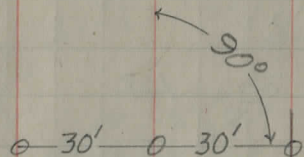
# GEORGIA ROAD

Burton Twp.

10455.8  $\Delta = 0^{\circ} 42' L - 0^{\circ} 0.0'$



1+00



Sept. 25, 1930

rain W.C. Marks

D. Parks

R. Hassel

T. Snyder

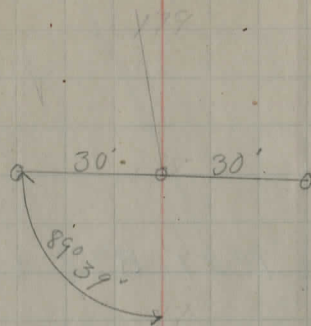
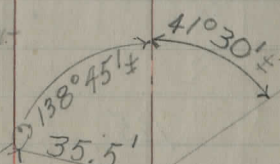
E. Parks

W. side 6"  
Evergreen

TAVERN

1010.1

E. root 18" Maple



Note: new refs see  
£ 1964 Data  
Pg. 48

35+00

$\Delta = 1^{\circ} 21' R$

31+30

$\Delta = 0^{\circ} 00'$

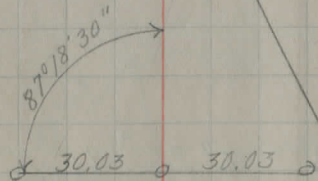
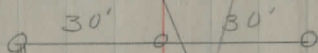
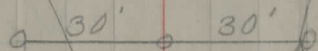
stopped Sept, 26, 1930

SHOWERS 750

14126.95

$\Delta = 5^{\circ} 23' L$

D. Parks  
R. Haese  
T. Snyder  
E. Parks



stopped Oct 3, 1930  
40+47

D. Parks  
R. Hassel  
T. Snyder

Stopped Sept, 27, 1930

D. Parks  
R. Hassel  
T. Snyder

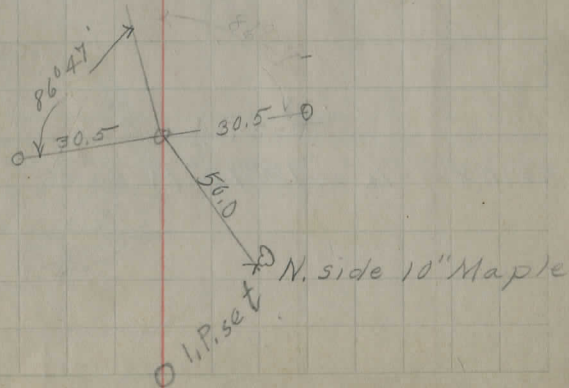
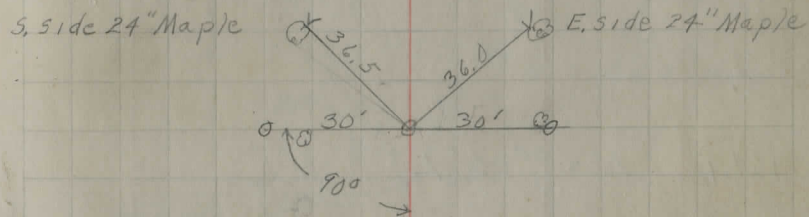
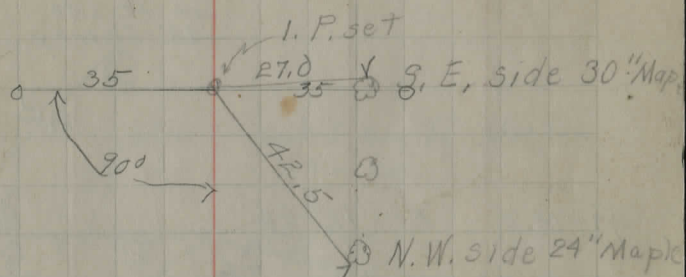
Cold, West Wind 70°

31+31.05  $\Delta = 1^{\circ}26' R$

14+80  $\Delta = 6^{\circ}26' L$

10+55.8

$\Delta = 0^{\circ}00'$

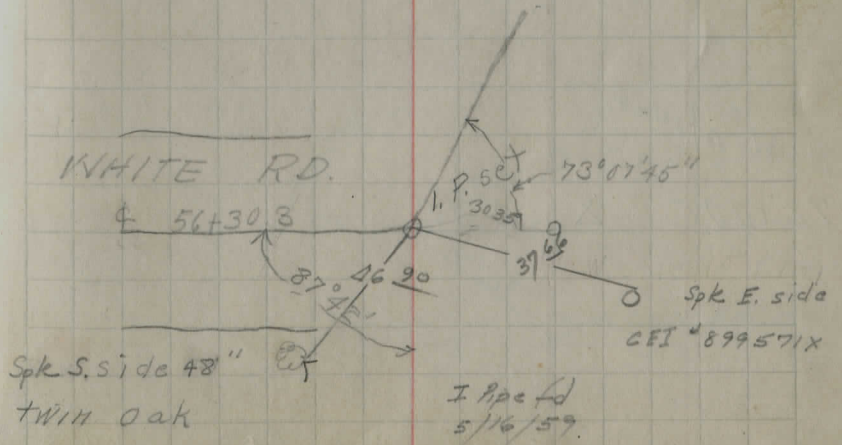
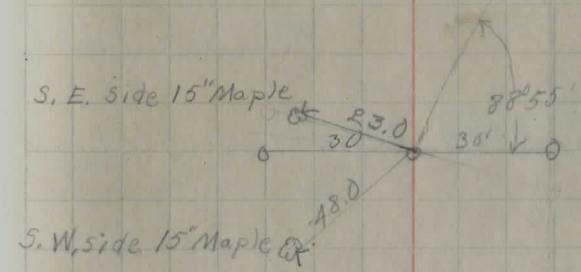




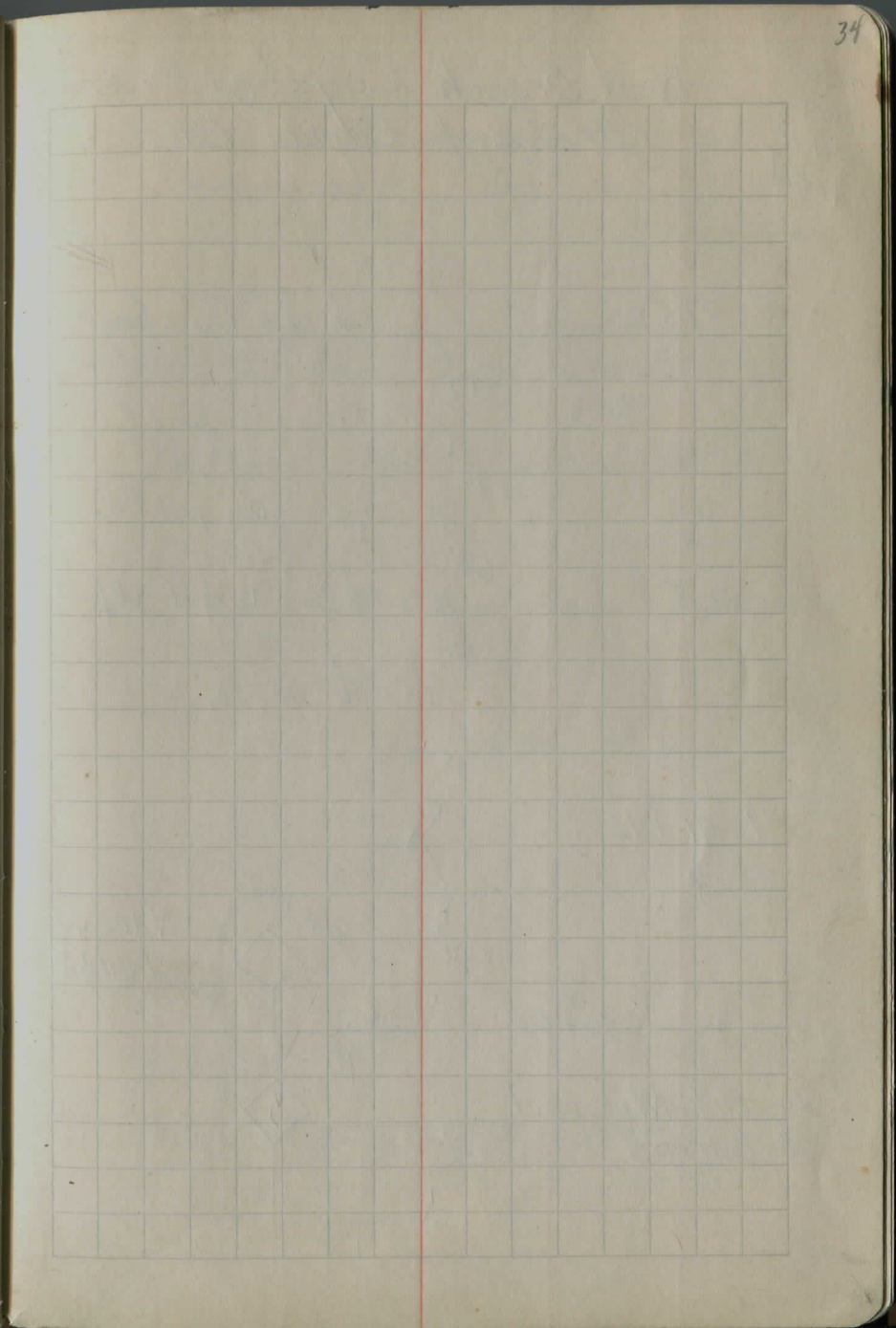
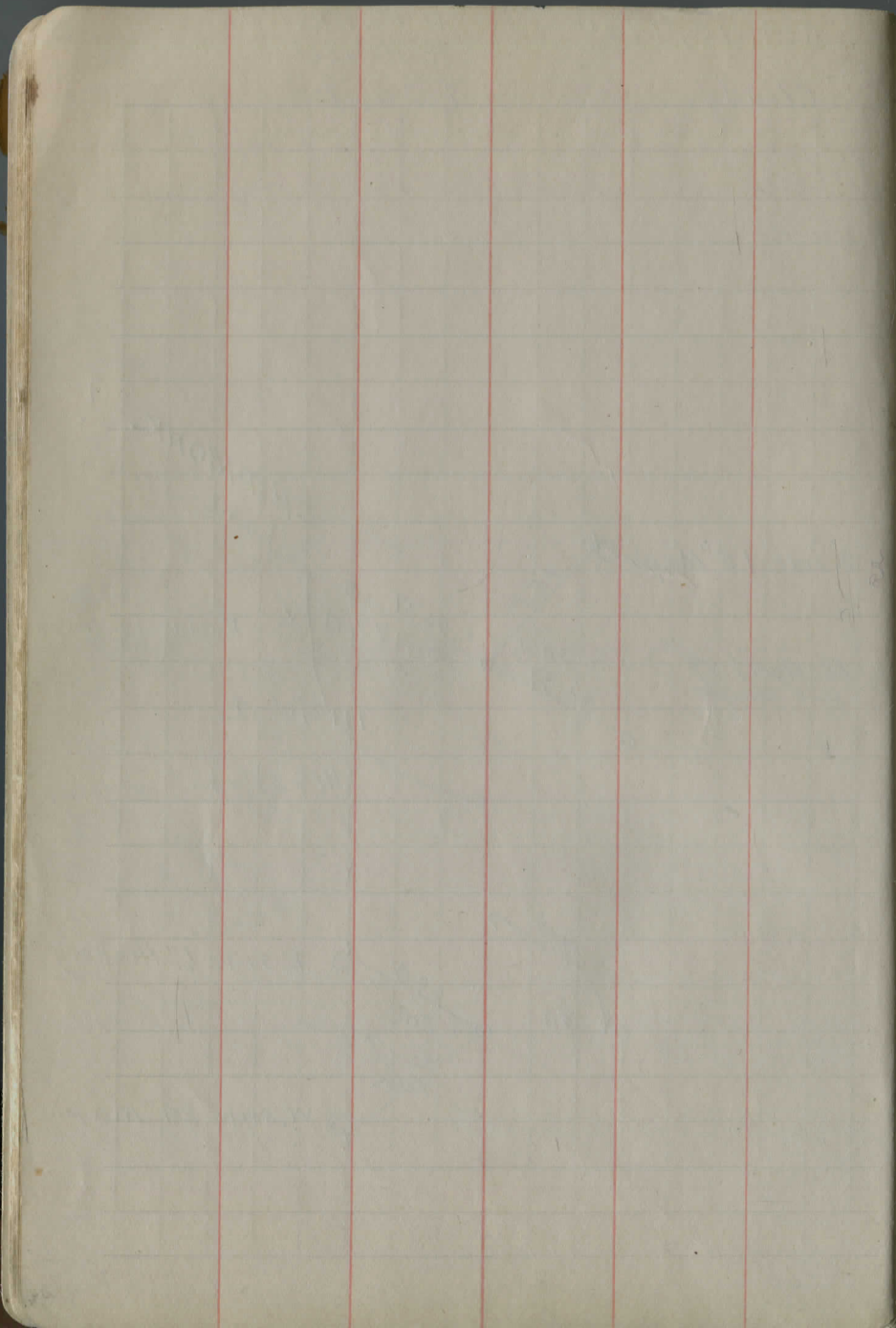
Stopped Oct. 4, 1930  
cool clear 65°  
D. Parks  
R. Hassel  
T. Snyder

63+63.1 Δ = 2° 10' R

56+30.3 Δ = 16° 52' 30" R side Rd. &



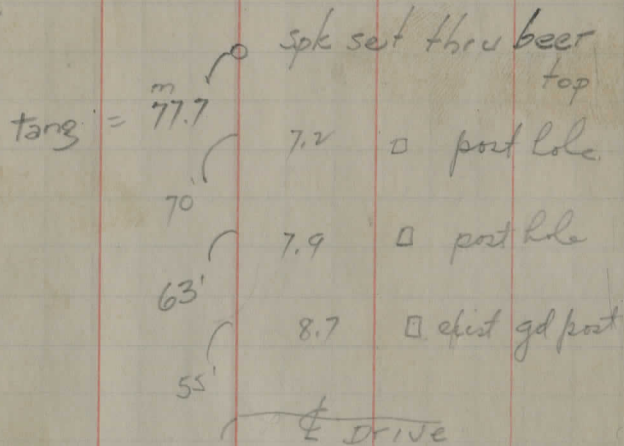




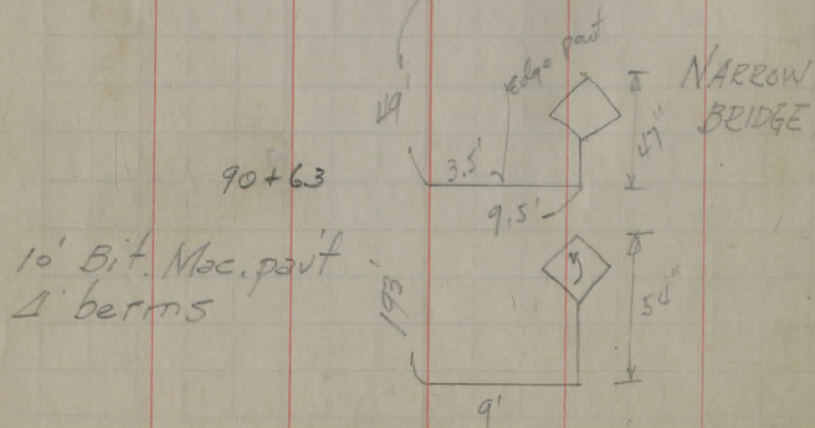
7-30-51  
 Pom  
 Ed B  
 Art T

West Branch Cuyahoga River  
 Bridge & Rapids Rd

91+87.5



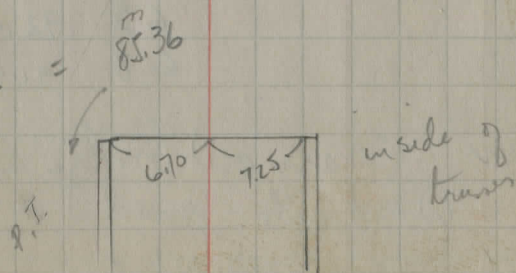
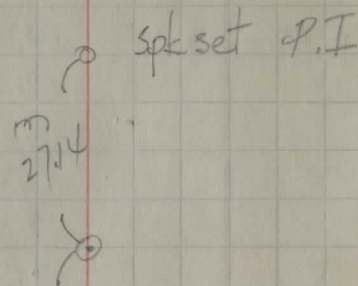
P1 91+12



87+0 with eye at 4-6  
 can see top 2" narrow  
 bridge sign

88+0 can read <sup>both</sup> words of  
 narrow bridge sign at 4-6"  
 up

80+35 on & can see sign



BM	7.16	107.16		100.00
T.P.	I.P. at PI		11.13	96.03
91			10.76	96.90
90			3.7	103.5
+50			1.8	105.4
89			1.0	106.2
88			0.3	106.9
T.P.	8.52	115.24	0.47	106.69
87			7.3	107.9
86			6.3	108.9
85			4.6	110.6
84			1.1	114.1
T.P.	4.63	98.66		96.03
91+87.5	P.T.		5.0	93.7
	H <sub>2</sub> O top		14.7	84.0
	FL ±		17.3	81.4
92+56			5.5	93.2
92+72.95	PC		6.2	92.5
93+0			7.0	91.7
94			7.6	91.1
95			6.3	92.4
96			6.8	91.9

94+00 Narrow bridge sign  
West side road  
± 10' off ±

36  
Spk SE side 1<sup>st</sup> tel pole S  
of Bridge

Levels Rapids Rd in  
vicinity of Cayahoga River  
7-30-51

Varsity Iron Works  
Clev. O

Misc. info Cuyahoga.  
River Bridge on Rapids Rd

Overall length of truss = 63.0  
clear span = ± 59.3

---

End support of truss = built up box  
girders of 2 - 6"x2" channels

S.E.



BM	0.00	100.00	100.00
			7.87 94.13
			8.07 91.93
			6.33 93.67
T.P.	5.08	97.01	8.07 91.93 ✓
			10.24 86.77
			11.91 85.10
			6.68 90.33
			7.82 89.19
			9.80 87.71
			4.87 94.14
			4.95 94.06
			3.84 93.17
BM set			6.76 90.75

Temple  
Pom ↑  
rod

39

Spt SE side 1<sup>st</sup> tel. pole S of bridge  
 W end S bridge seat  
 E " " " "  
 E part 91+87.5

Hub A

T.P. B

Hub C

D

E

W end N seat

E " " "

E rd on line of backwall seat

Spt NW side quad. elm N bank of Cuyhag  
 ± 40' E of bridge

8-20-51 CH 21

BM,	H.	ES	ELV
Hub C	9.01	98.76	
	←	Sta	
	00°	115'	8.6 89.4
	25-57	116'	5.0 88.0
	345-05	117'	6.2 86.8
	339-01	125'	9.1 83.9
	309-01	83'	3.2 89.8
	309-01	96'	6.4 86.6
	309-01	102'	6.4 86.6
	309-01	112'	3.8 89.2
	309-01	136'	3.6 89.4
	309-01	141'	9.1 83.9
	267-24	147'	3.0 90.0
	267-24	160	3.8 89.2
	267-24	162	9.1 83.9
	25-33	41	5.4 87.6
	215-40	61	4.7 88.3
	215-40	86	5.8 87.2
	215-40	135	4.5 88.5
	177-38	186	4.0 89.0
	177-38	142	5.6 87.4
	164-39	115	5.6 87.4
	164-39	192	4.3 88.7
	152-26	192	5.4 87.6
	152-26	171	4.1 88.9

40

Spk NW. side Quad Elm N Bank River 40' E of Bridge (P 39)

H.I above Hub C + 45"

pt on BS on W. edge of Bay in Canal

S. Bank

Mouth of Bay - West Bank of River

Water Level @ Bay Entrance - West Bank of River

W Bank of River

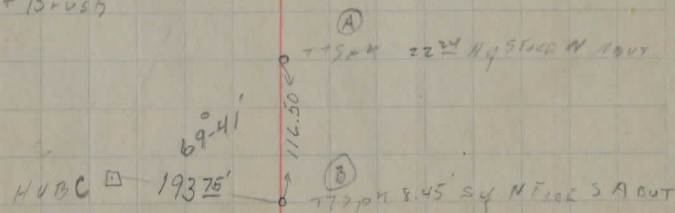
Water level River

W. Bank River

Water level - pt. River 1/2 mi to East

± Prolong. of West Bank S. by ± Prolong. of Canal W. by L to Bridge

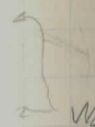
edge of Brush



	HI 93.00		ELV
←	Sta.	Red.	
152-26	88'	5.8	87.2
152-26	61'	5.3	87.9
129-01	64'	6.6	86.4
129-01	140'	5.2	87.8
121-42	133	6.4	86.6
42-09	46'	5.1	87.9
52-30	95'	4.3	88.7
52-30	140	3.5	89.5

Bridge levels Sabut 9-12-51

BM	7.88	✓ 95.63	90.75
top wheel stone			93.75
seat level			92.08



Wet Weather from S.W. Marsh land Westerly & Northerly  
 Grnd up fast S & E

UP Steep South

98.63  
 93.75 ✓  
 4.88

98.63  
 92.08 ✓  
 6.55

3 by jans  
 100' riffer  
 here other  
 margins

18" Frontg on river

BM 2.09 92.84 90.75

truss bearings  $\frac{92.84}{92.10}$   
0.74

West bear 0.73 92.11

East " 0.73 92.11

Intermediate steps (conc) 0.75 92.09

21.0 %

$\frac{92.84}{92.08}$   
0.76

BEARING PLATES

SO ABUT.

9-18-51

10-11-51

CUYMOS BRIDGE CH #1 HERRITT & MAYNARD

BM.	6.81	97.56	90.75
		5.50	92.06
		5.49	92.07
		5.50	92.06
		5.51	92.05

SPAN NW Side Quad Elm N. Bank Cuyamoc 240' E of Bridge

NE of Bridge SEAT  
 SEE " "  
 SW R " "  
 NW

H. Patterson	Rapids Rd	X Sec.	5-10-58
P. Young			
D. Parks	+	HI	- Elev
B.M.	5.57	96.32	90.75
T.P.	4.22	96.53	4.01 92.31
97+0			92.90
96+0			92.08
95+0			92.38
T.P.	3.48	95.84	4.17 92.36
94+0			91.33
93+0			91.71
92+0			94.03
B.M.	11.11	101.88	5.07 90.77
91+0			96.61
T.P.	9.66	110.73	0.81 101.07
90+0			102.68
89+0			106.13
T.P.	6.09	113.15	3.67 107.06
88+0			104.06
87+0			107.95
86+0			109.00

49

SPK. NW. side	Quad.	Elm	N. Bank	River	± 90	E. Bridge						
Water	W	7.70	3.80	3.63	3.64	4.05	6.74	4.65	out			
		30	24	14	5	9	15	20				
Water		10.60	5.12	4.60	4.45	4.41	7.44	5.93	out			
		29	24	11	1.5	6	13	22				
out		7.24	4.93	4.36	4.11	4.15	4.41	6.10	7.21	out		
		50	32	19	12	3	12	18	25			
Water		8.70	6.73	5.45	4.85	4.46	4.51	4.66	4.41	7.50	7.05	out
		38	33	22	12	2	4	7	14	20	25	
Water		10.40	5.21	4.50	4.13	4.13	4.35	4.70	8.65	7.20	W.P. #	out
		25	22	14	4	4	8	11	20	25		
					1.83	8.00	1.50					
					7	1.81	8					
		12.70	9.32	7.72	6.10	5.52	5.27	5.52	7.94	8.58	6.32	up
		50	32	19	12	6	7	7	14	19	25	Stoop
out		2.88	2.92	2.70	2.23	2.01	2.31	2.66	3.16	out		
		50	28	8	6	11	5	14	19	30		
out		2.27	6.30	4.96	4.60	4.55	4.78	6.90	2.77	out		
		50	25	11	6	11	4	12	18	30		
		5.85	5.50	7.16	6.30	6.09	6.01	6.40	7.82	7.12	5.31	
		50	30	10	6	11	3	12	16	19	30	
		7.03	6.83	6.30	5.42	5.22	5.24	5.54	6.65	6.67		
		50	25	12	6	11	2	10	14	30		
		6.49	6.86	5.15	4.51	4.15	4.23	5.40	5.93			
		50	28	13	8	11	8	13	30			

	+	HT		Elev
85+0		113.15		110.50
T.P.	11.40	124.53	0.02	113.13
84+0				113.59
83+0				118.39
T.P.	10.88	135.10	0.31	124.22
82+0				124.54
81+0				130.60
T.P.	0.27	124.95	10.42	124.68
T.P.	0.57	114.05	11.47	113.48
T.P.	0.08	102.89	11.29	102.76
B.H. TP	9.78	102.60	10.02	92.82
B.M.			11.84	90.76

Stakes were set 25' off Tang &  
 on E. side Rd.  
 □ = Tang &  
 ○ = Occ. &

	V		E	
	$\frac{4.98}{50}$	$\frac{4.65}{28}$	$\frac{3.52}{12}$	$\frac{3.01}{9}$
				$\frac{2.65}{7}$
				$\frac{2.68}{8}$
				$\frac{3.36}{13}$
				$\frac{3.30}{19}$
				out
out	$\frac{10.90}{29}$	$\frac{12.30}{13}$	$\frac{11.02}{8}$	$\frac{10.90}{7}$
				$\frac{11.10}{8}$
				$\frac{12.20}{12}$
				$\frac{10.15}{18}$
				$\frac{9.90}{24}$
				out
	$\frac{3.36}{50}$	$\frac{3.30}{18}$	$\frac{8.00}{12}$	$\frac{6.58}{8}$
				$\frac{6.14}{7}$
				$\frac{6.30}{7}$
				$\frac{7.15}{12}$
				$\frac{2.20}{20}$
				$\frac{2.5}{30}$
				out
	$\frac{9.67}{50}$	$\frac{9.15}{20}$	$\frac{12.60}{15}$	$\frac{11.60}{8}$
				$\frac{10.56}{7}$
				$\frac{10.30}{7}$
				$\frac{11.35}{12}$
				$\frac{8.67}{19}$
				$\frac{7.53}{25}$
				out
	$\frac{3.00}{56}$	$\frac{3.28}{30}$	$\frac{3.70}{17}$	$\frac{6.20}{14}$
				$\frac{4.55}{7}$
				$\frac{4.50}{7}$
				$\frac{4.90}{9}$
				$\frac{5.60}{15}$
				$\frac{2.25}{19}$
				$\frac{1.25}{30}$
				out

# Final Alignment White Rd

Side stakes set 30' off &  
East side Rd.

40+05.70

26+30.55

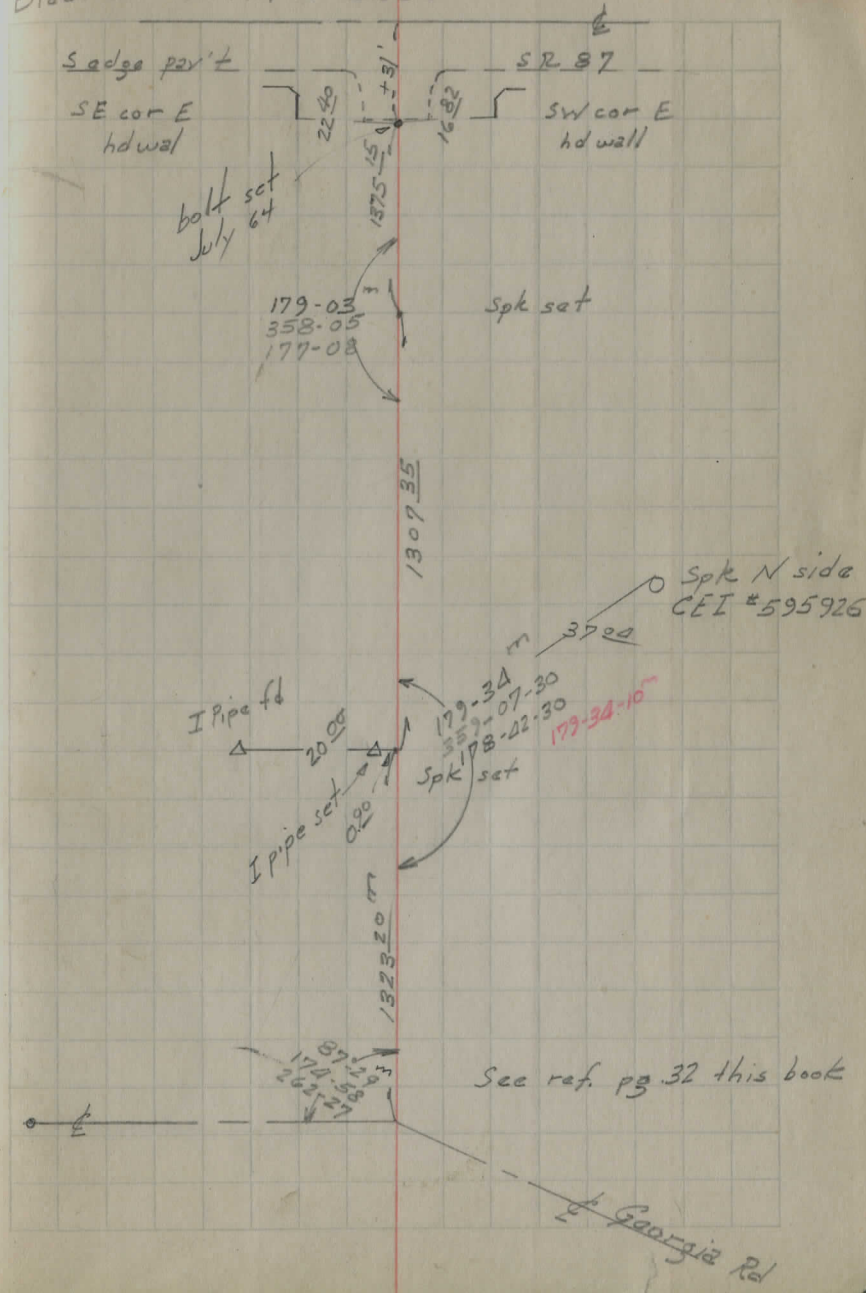
13+23.20

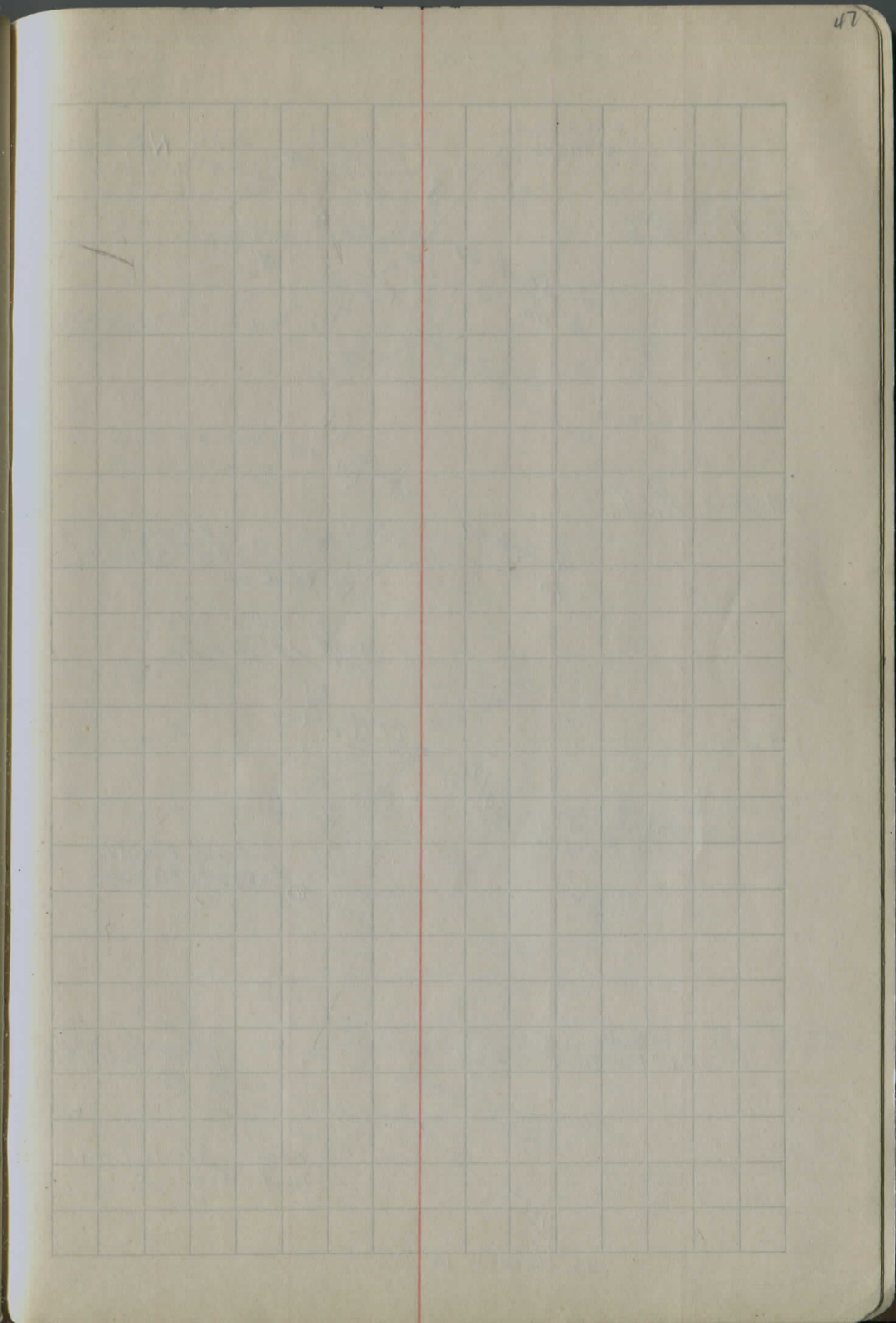
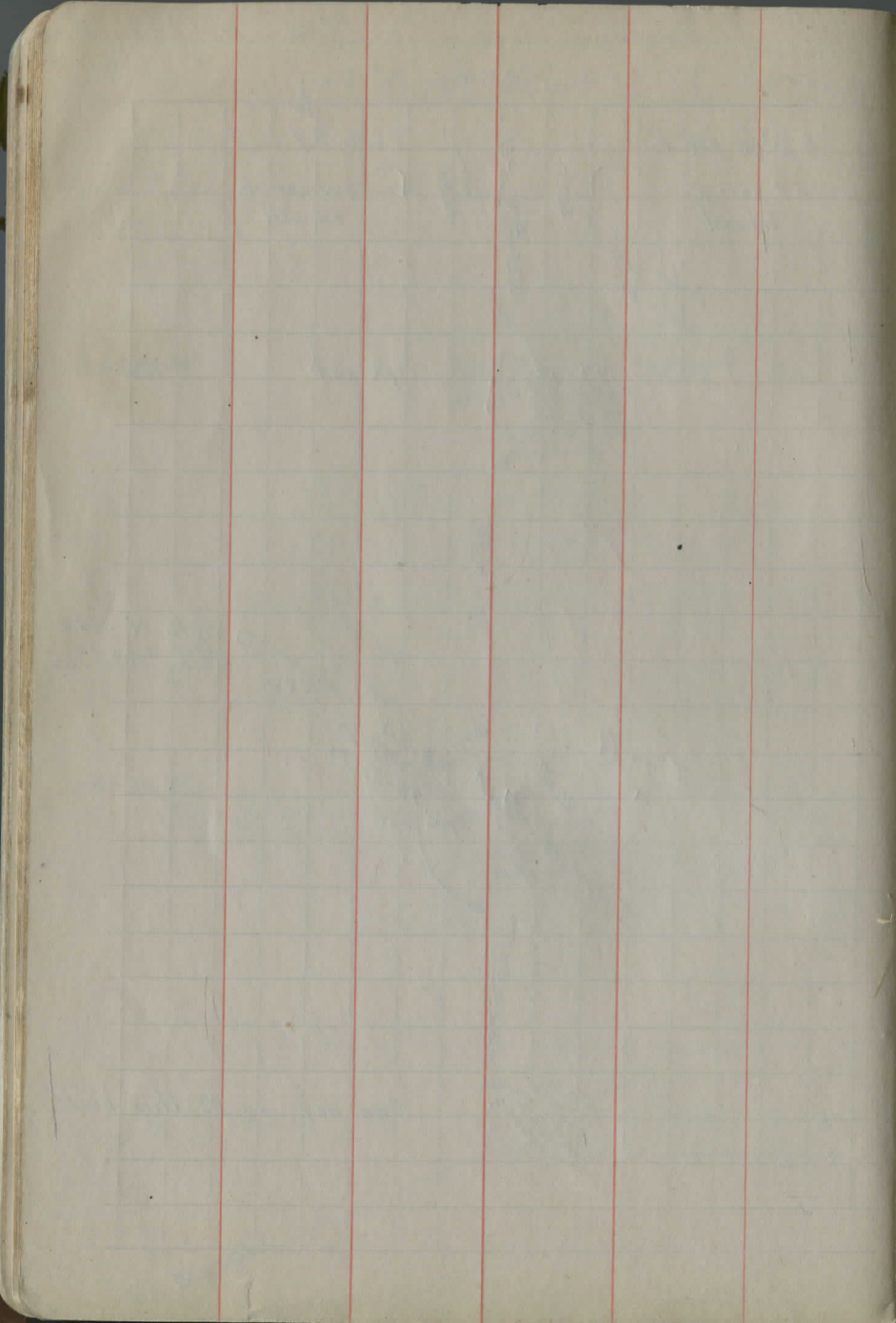
Spk Set  
.10 ft west  
of Random  
point  
P.O.T. June 1959

I pipe set July 64  
SPK Set  
.90 ft west  
of Random  
point  
P.O.T. June 1959

1375.14  
1307.38  
1329.26  
4005.70 040

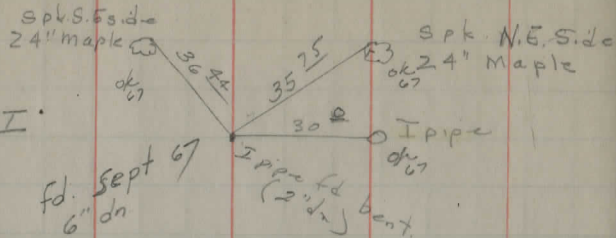
Temple Patterson Diadrach Random Line White Road May 18, 1959 46



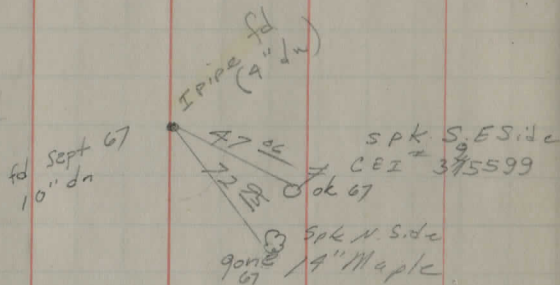


Georgia Rd

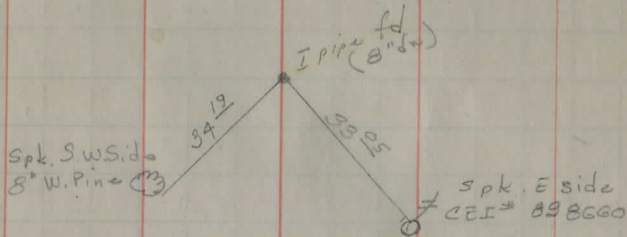
31+31<sup>05</sup> P.I.



14+80 P.I.



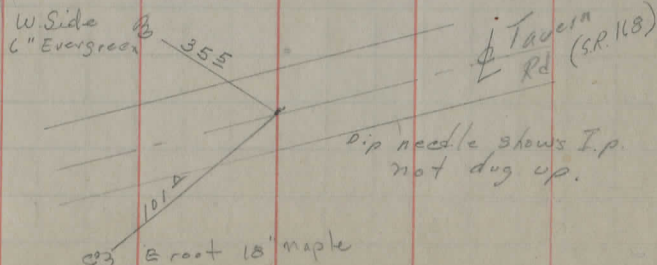
1+0



W. Side 6" Evergreen

Tavern Rd (SR 118)

dip needle shows I.p. not dug up.



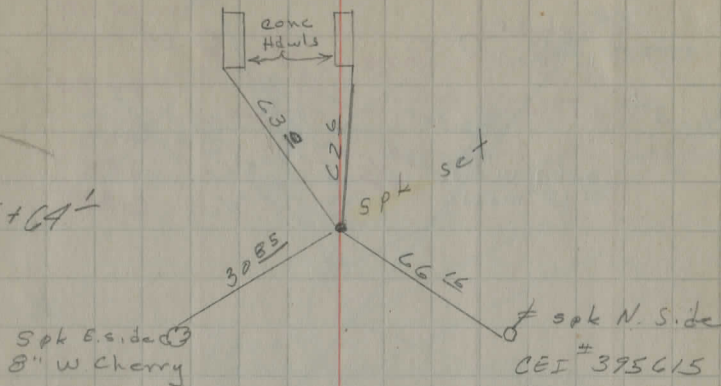
0+0

Davidson  
Raney  
Young

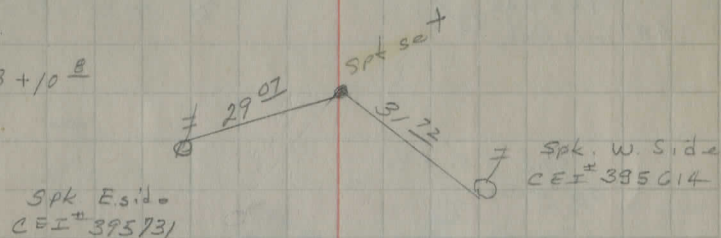
12/22/64

cldy - cool -

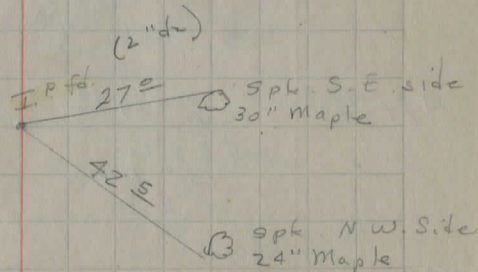
45+64



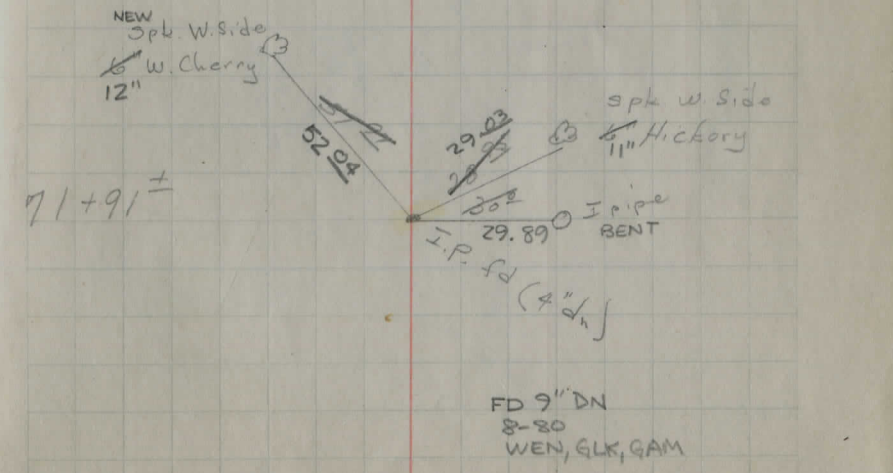
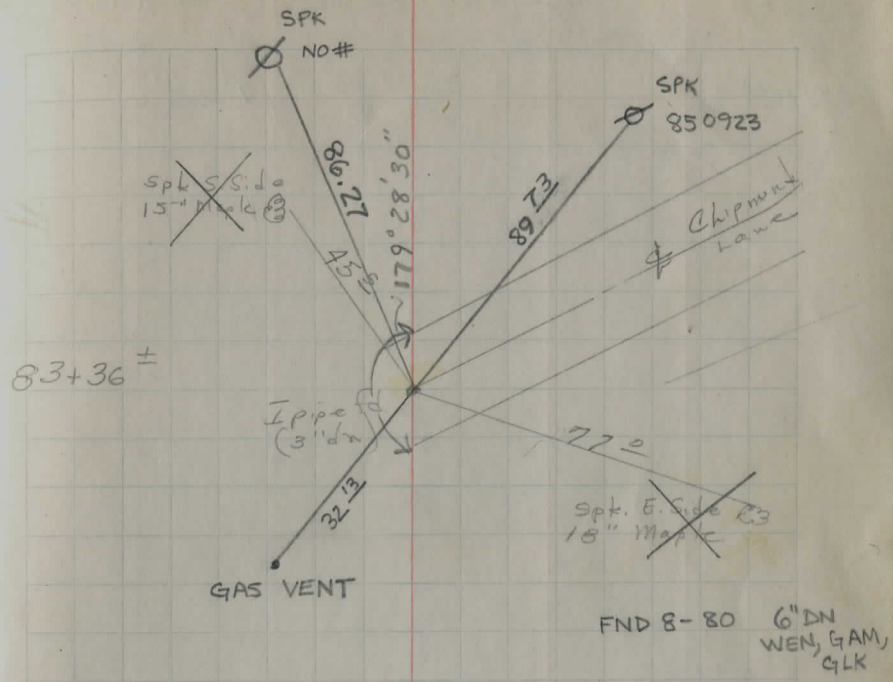
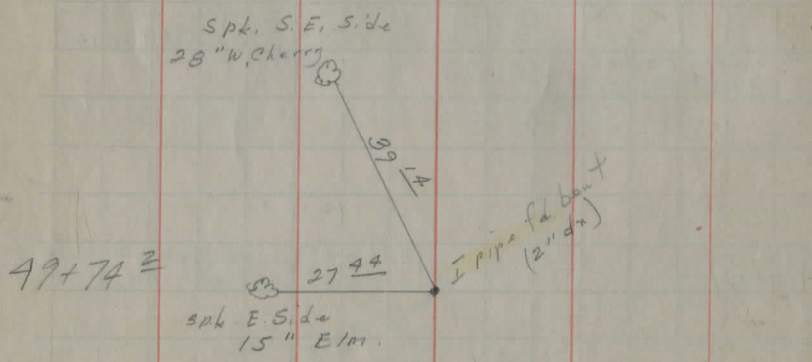
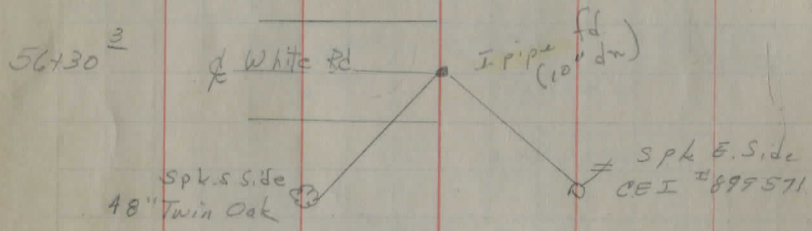
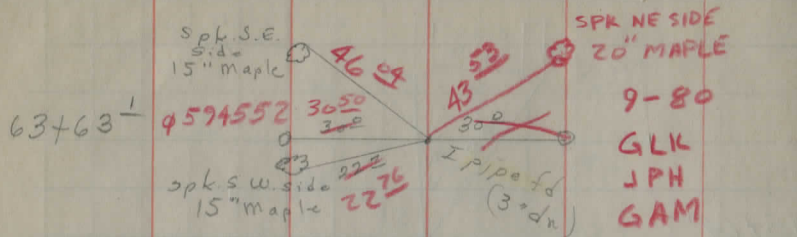
43+10



40+47



Georgia Rd.



H. Patterson  
P. Young

#133  
Chipmunk lane  
7-6-60

Vol D "Pg 32  
Rd. Records  
65' wide

P.O.T.

22400<sup>00</sup>

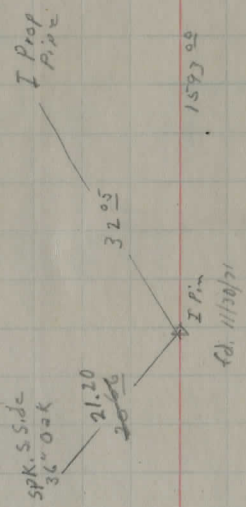
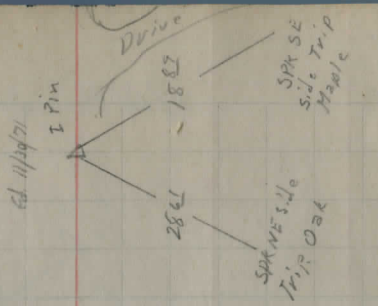
Stakes set 30' off & Both sides every 100'

6707<sup>00</sup>

P.O.T.

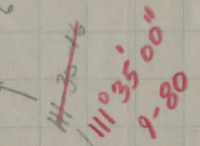
0+0

I PIN DISTURBED  
RESET 9-80  
KOVACH  
HAINES  
MOHNACSKY



I PIN DISTURBED  
RESET 9-80  
KOVACH  
HAINES  
MOHNACSKY

607<sup>00</sup>



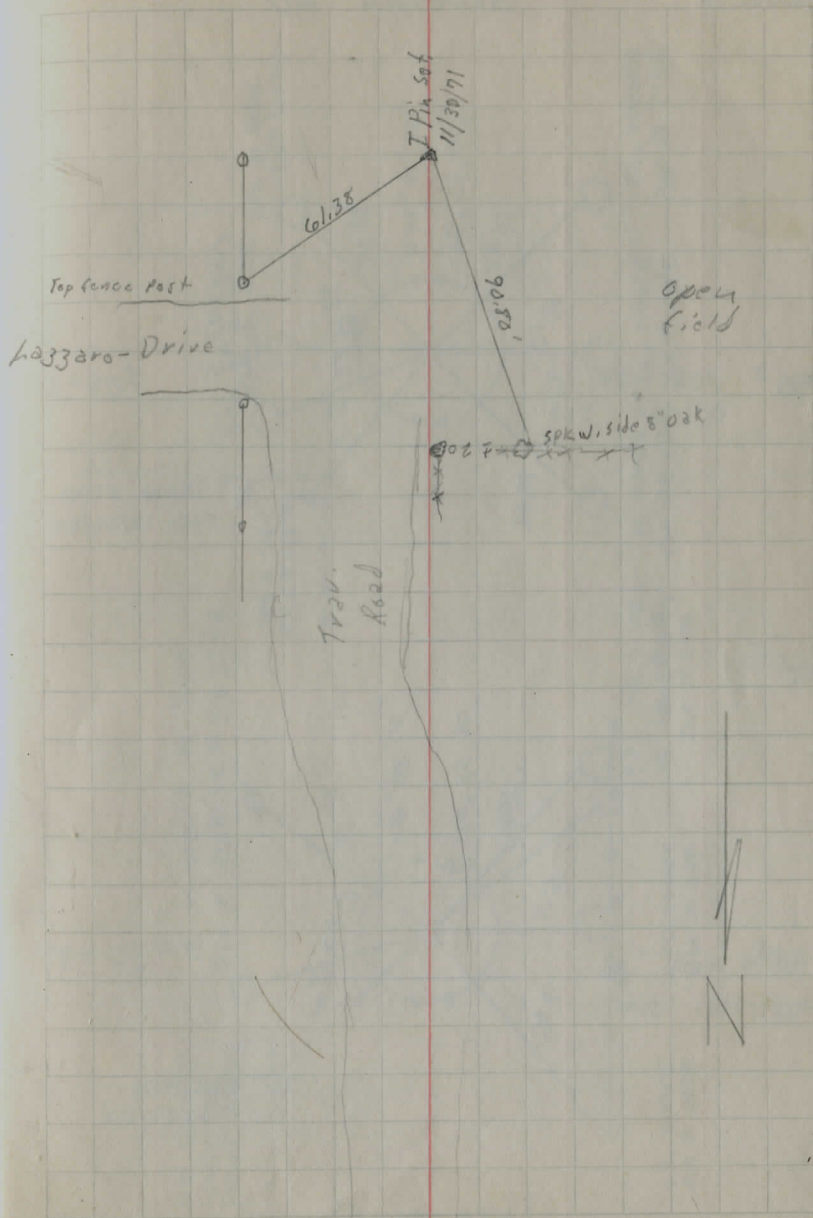
IPFD. Georgia Rd.

H. Patterson  
D. Wenzel  
J. Schuster

11/30/71 Snow 32°

38400

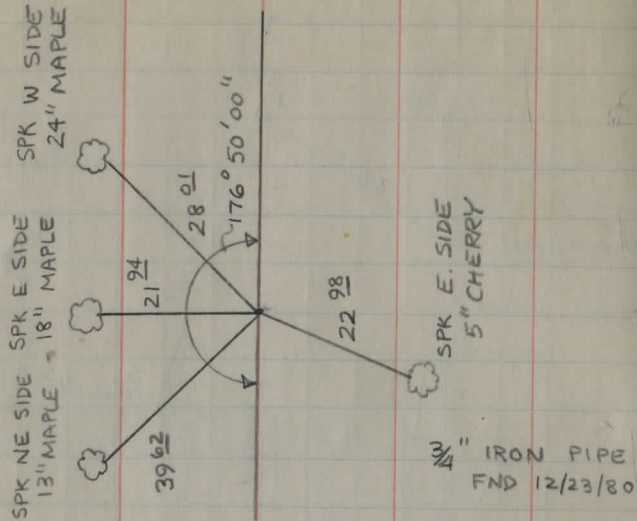
P.O.T.



W. NYMAN 12/26/80  
 G. MOHNACSKY  
 J. VARANESE

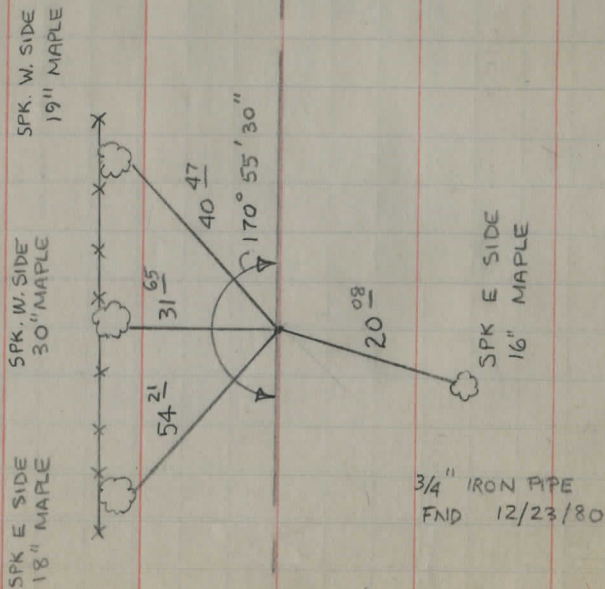
GEORGIA RD

PI  
 93+56±



3/4" IRON PIPE  
 FND 12/23/80

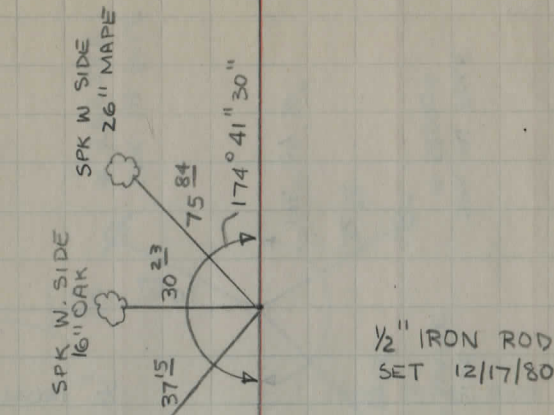
PI  
 87+48±



3/4" IRON PIPE  
 FND 12/23/80

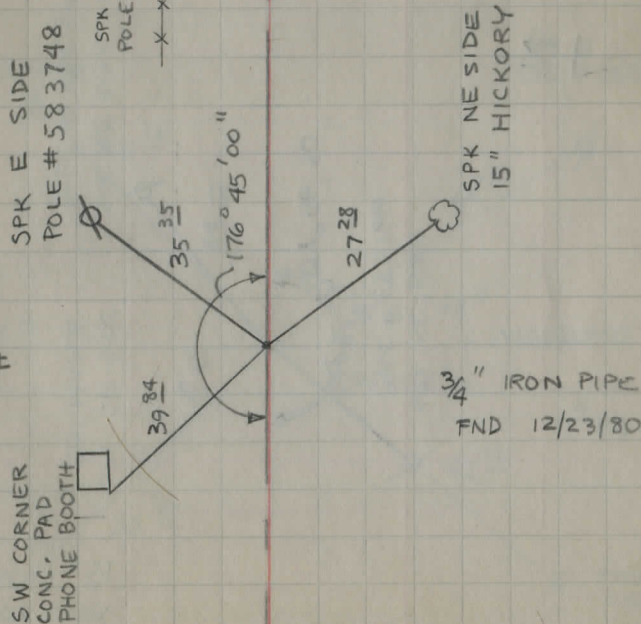
(SPK 8748)

PI  
 102+23±



1/2" IRON ROD  
 SET 12/17/80

PI  
 98+03±

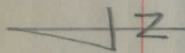
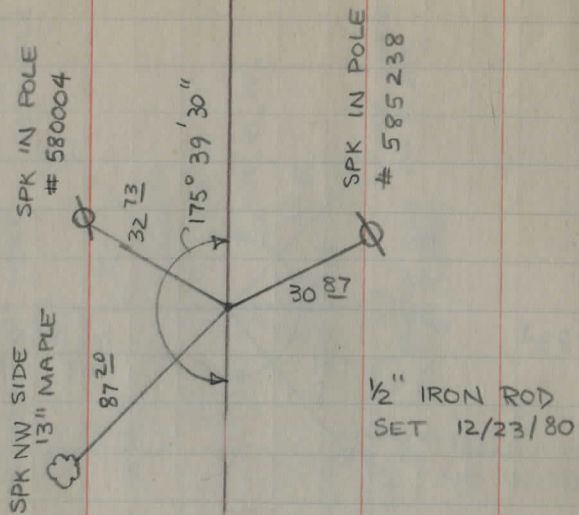


3/4" IRON PIPE  
 FND 12/23/80

SW CORNER  
 CONC. PAD  
 PHONE BOOTH

PI

126+23±



1/2" IRON ROD  
SET 12/23/80

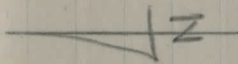
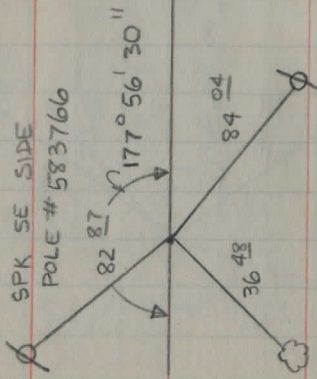
SPK NE SIDE  
POLE # 585237

SPK SW SIDE  
30" MAPLE

1/2" IRON ROD  
SET 12/23/80

PI

119+09±

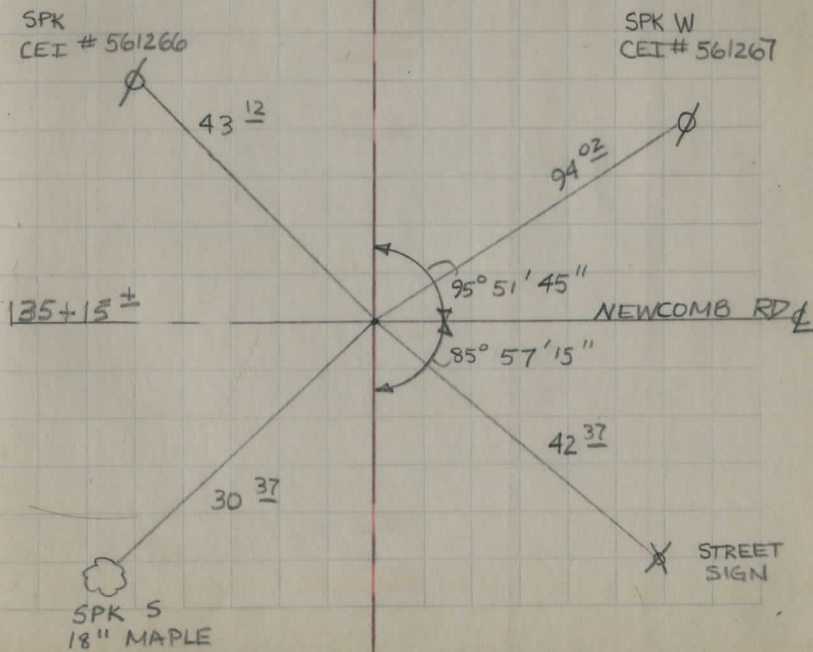


1" IRON PIPE END 25" DN 8/80  
3/8" X 10" NAIL SET ABOVE PIPE  
2" DN 12/16/80 REH, GAM, GLK

SPK  
CEI # 561266

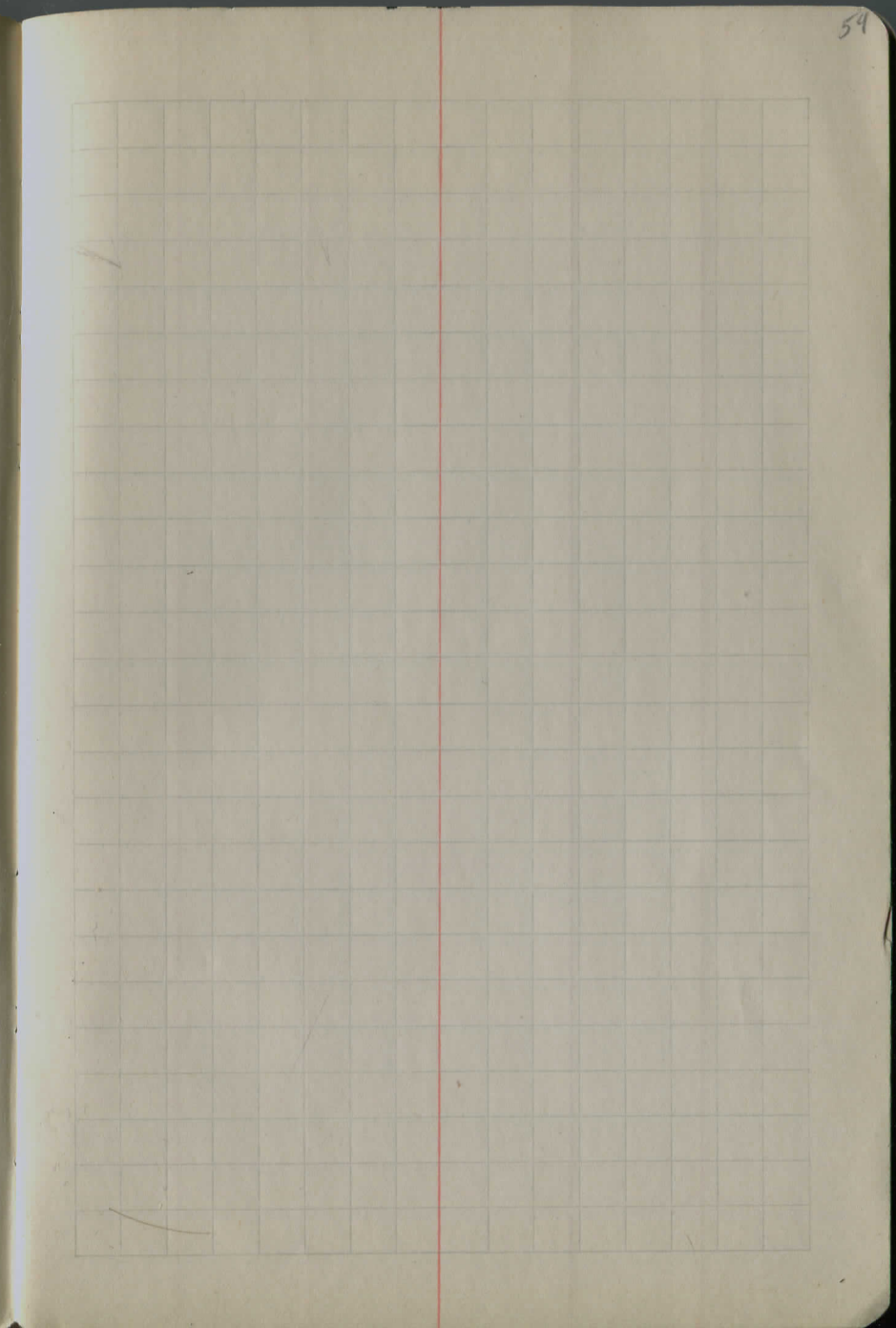
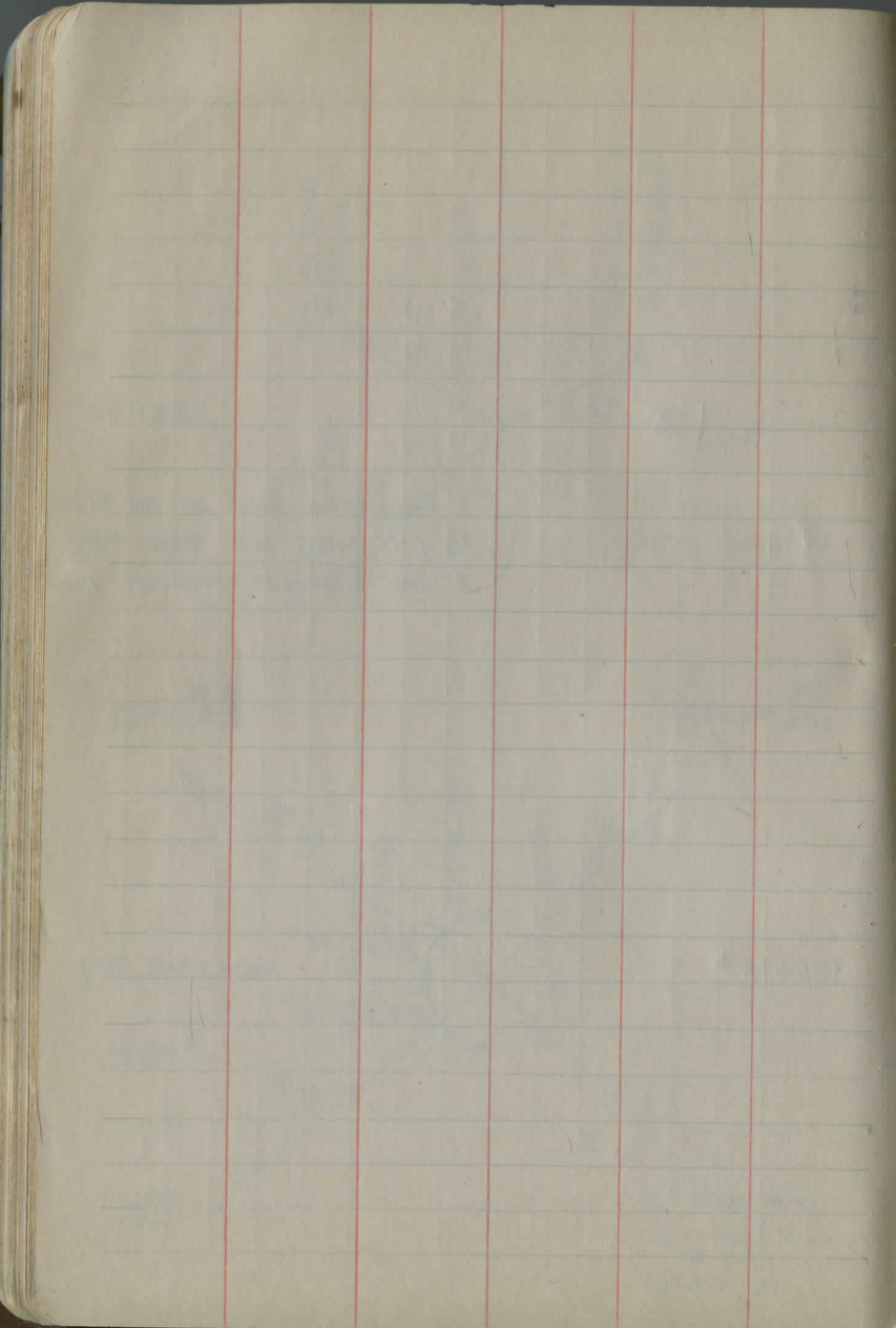
SPK W  
CEI # 561267

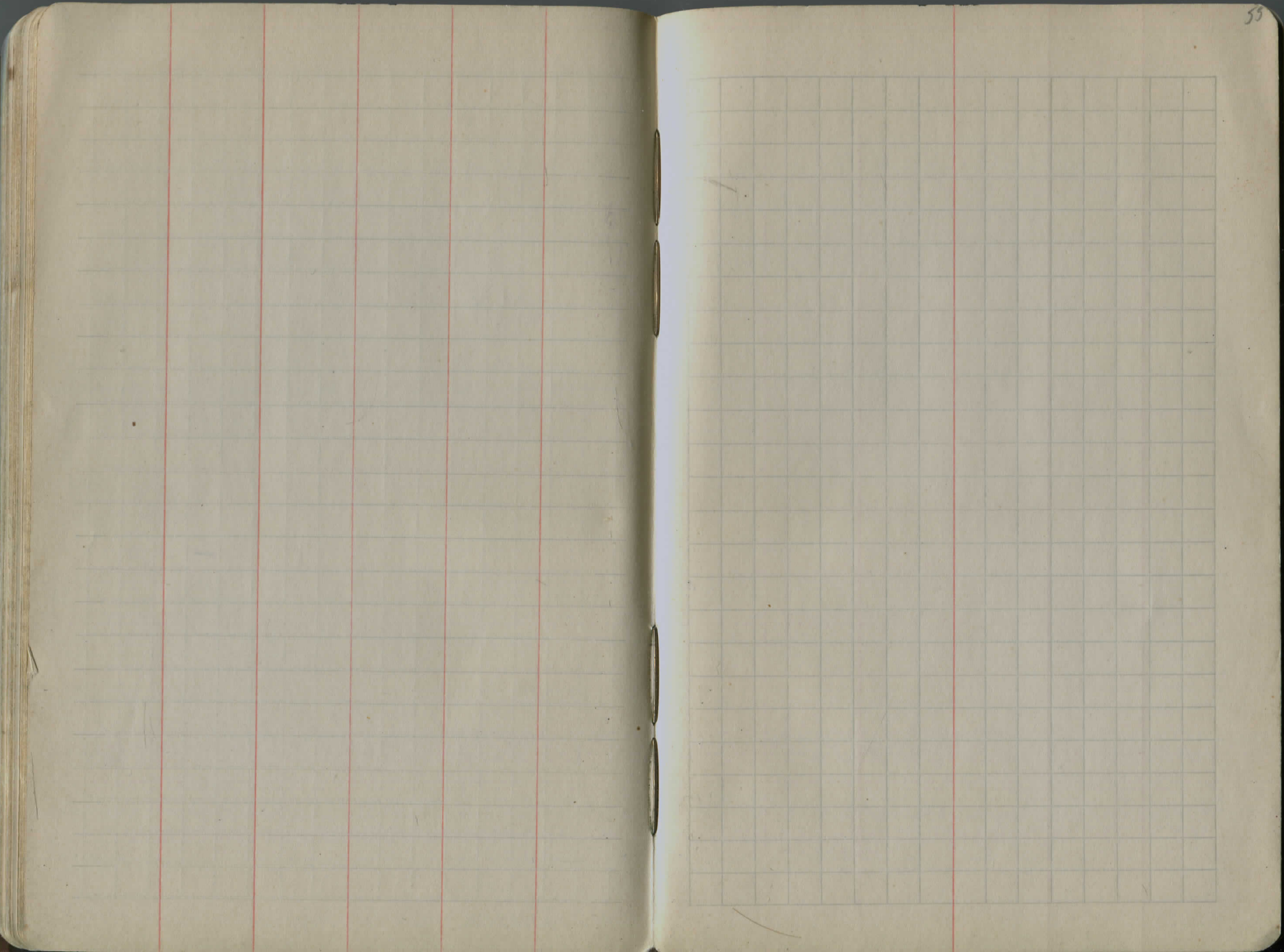
135+15±

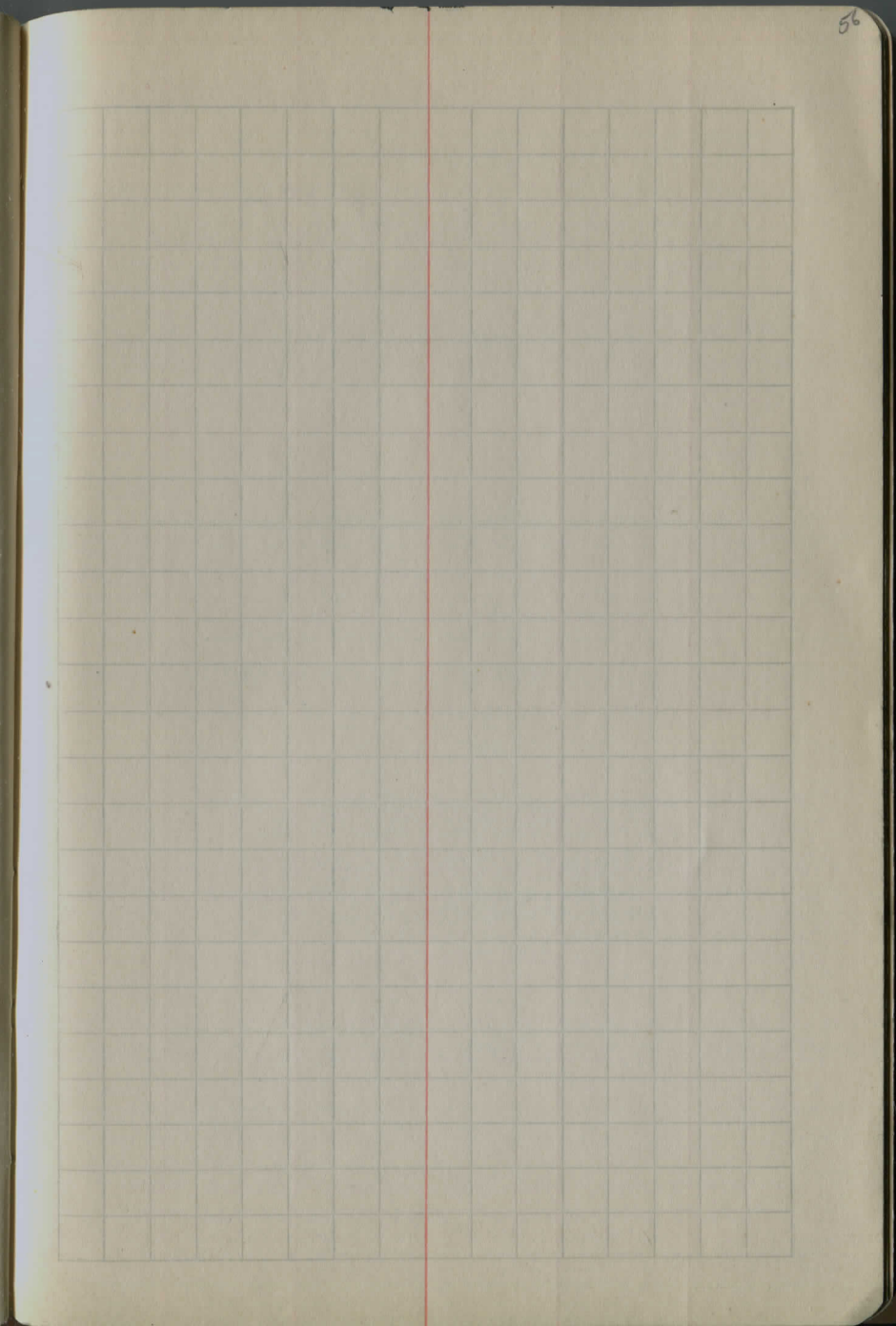
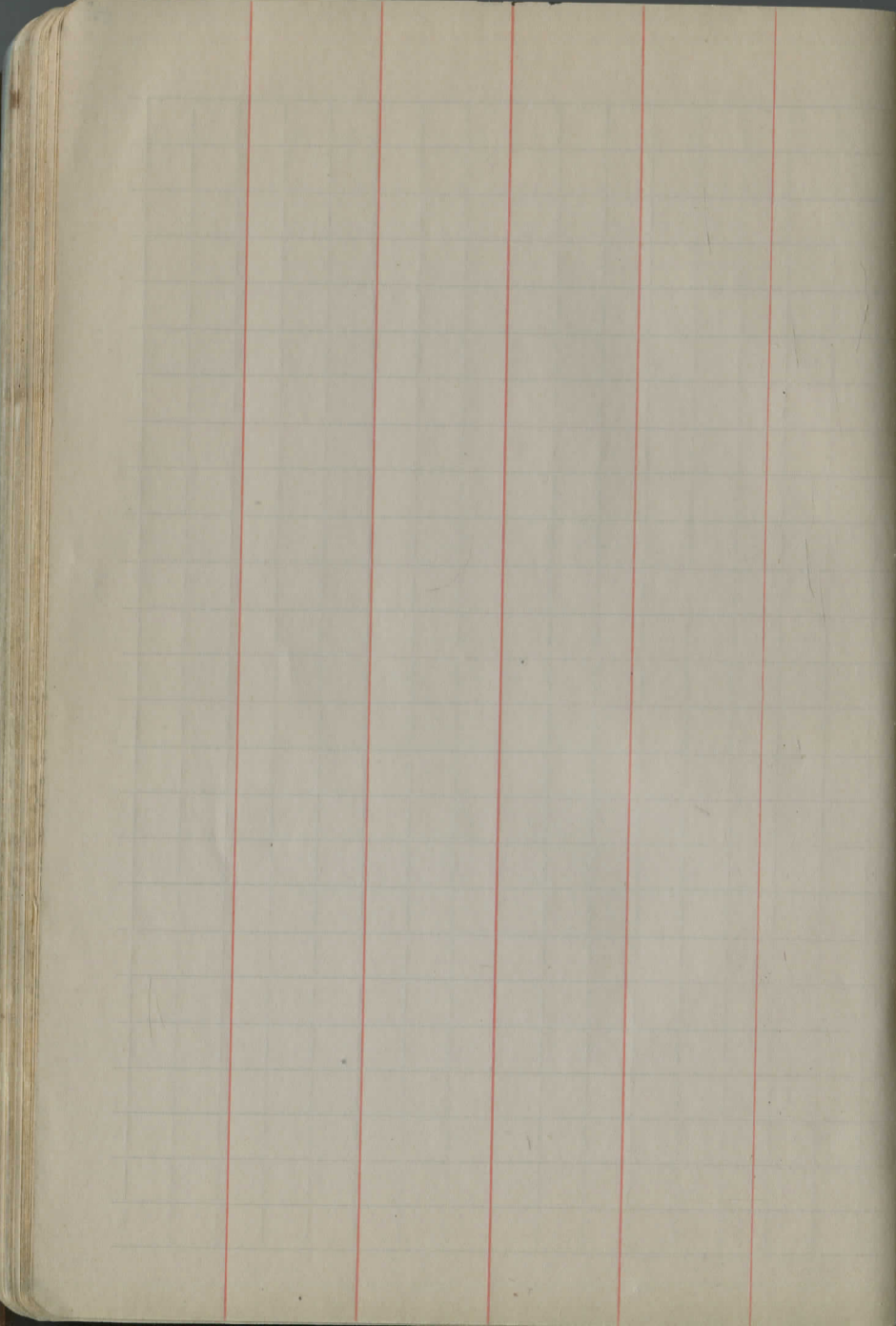


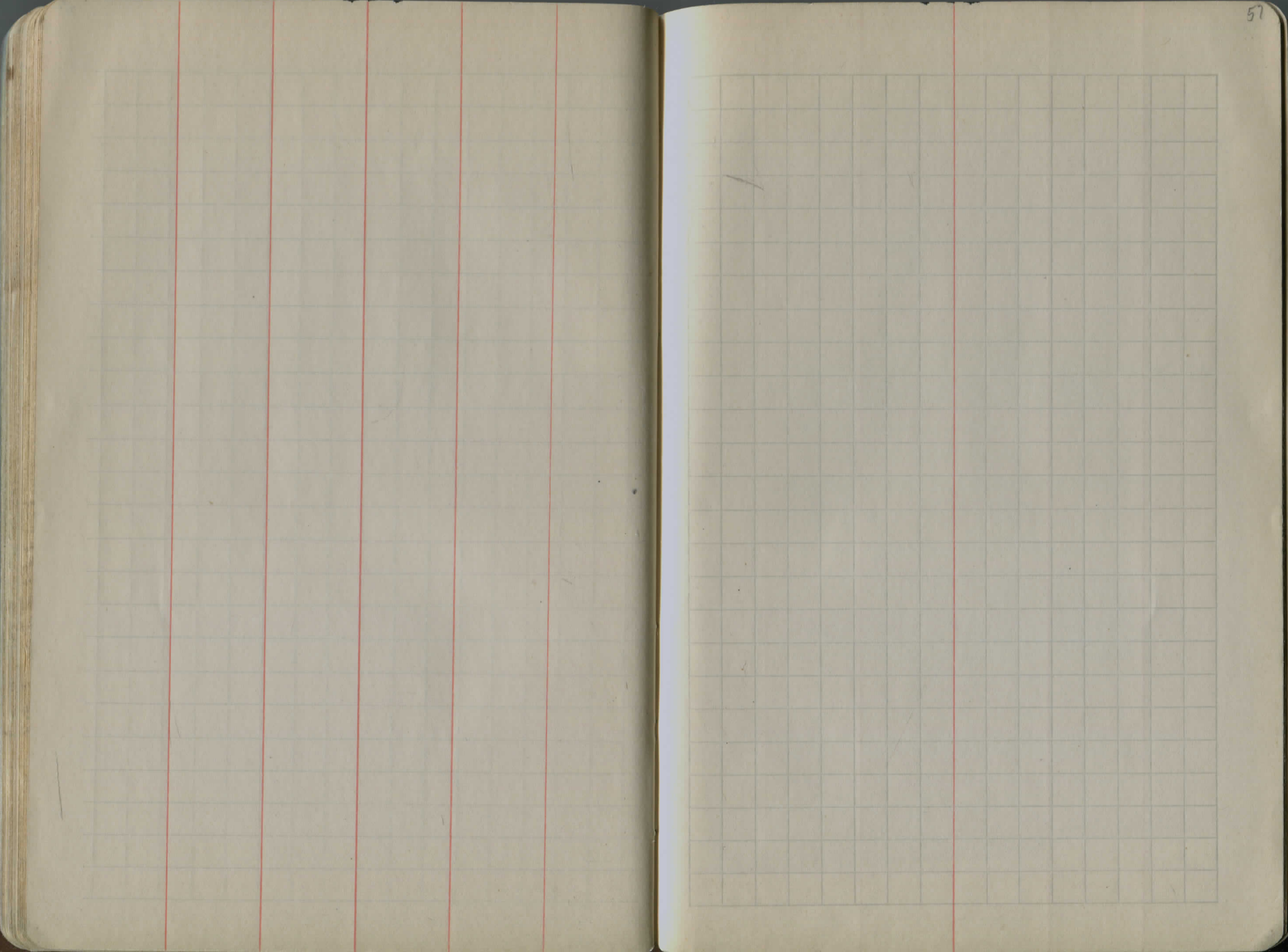
SPK S  
18" MAPLE

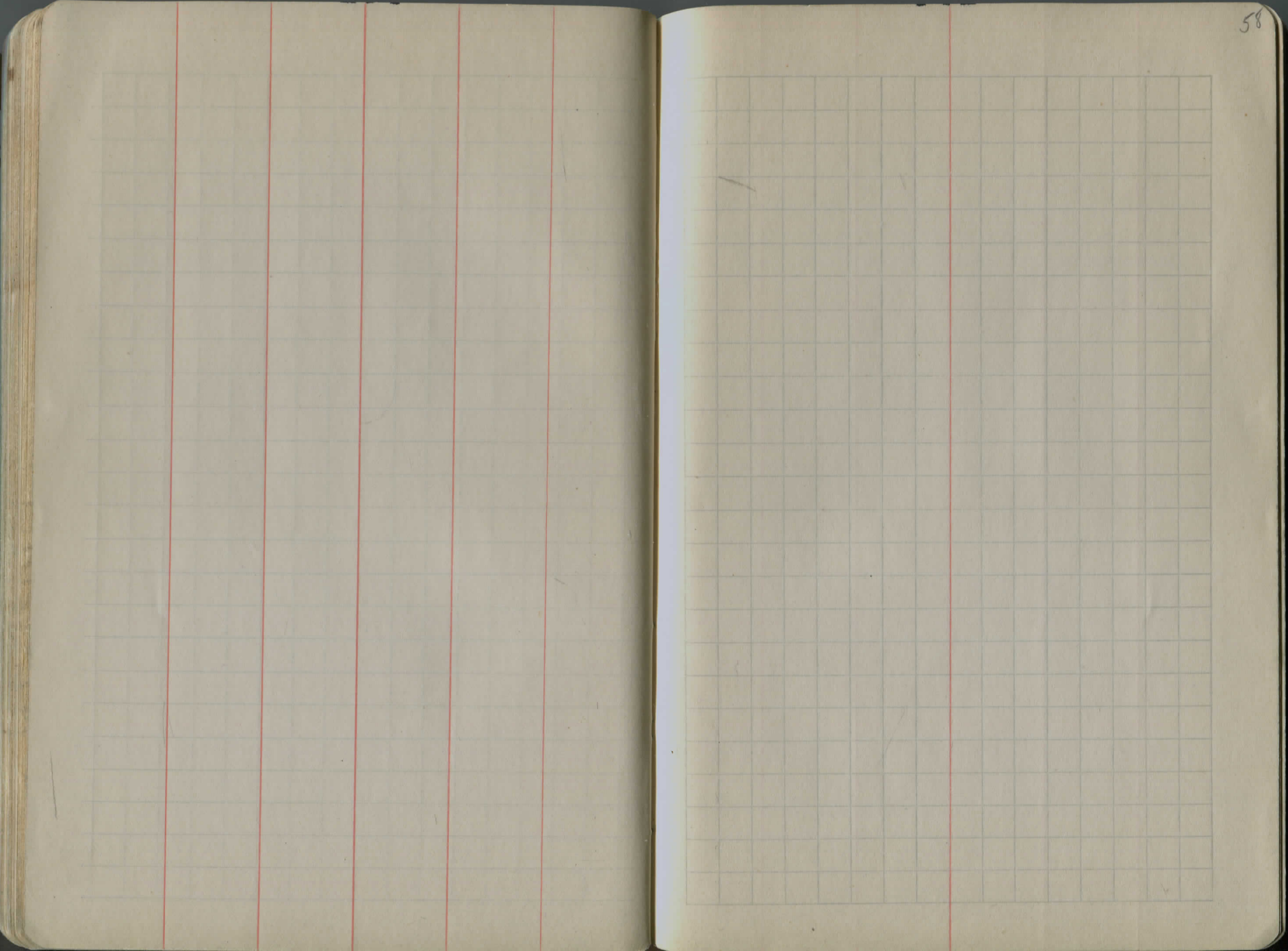
STREET  
SIGN











$$4) 44.07(11.02$$

$$\begin{array}{r} 4 \\ \underline{4} \\ 07 \end{array}$$

$$5) \frac{10}{40} (2667$$

$$4) 711.97(177.92$$

$$\begin{array}{r} 4 \\ \underline{4} \\ 31 \\ \underline{28} \\ 39 \\ \underline{36} \\ 37 \\ \underline{36} \end{array}$$

$$4) 14.16667(354.17$$

$$\begin{array}{r} 12 \\ \underline{12} \\ 21 \\ \underline{20} \\ 16 \\ \underline{16} \\ 6 \\ \underline{4} \\ 26 \end{array}$$

$$43.1$$

$$\begin{array}{r} 100.0 \\ \underline{143.1} \\ 36.82 \end{array}$$

$$179.92$$

$$\begin{array}{r} 143.10 \\ \underline{36.82} \end{array}$$

7 x 1/2

$$\frac{7}{12} \times 4 =$$

$$7 = 25.2$$

$$6 = 32.0$$

$$\begin{array}{r} 1.8 \\ \underline{1.8} \\ 144 \\ \underline{15} \\ 3.24 \\ \underline{7} \\ 9 \end{array}$$

$$\begin{array}{r} 1.25 \\ \underline{1.25} \\ 625 \\ \underline{240} \\ 125 \\ \underline{15525} \\ 7 \end{array}$$

$$\begin{array}{r} 100 \\ \underline{56.9} \\ 43.1 \end{array}$$

$$\begin{array}{r} 8) 10.85(1.36 \\ \underline{8} \\ 28 \\ \underline{24} \\ 45 \end{array}$$

$$359.88$$

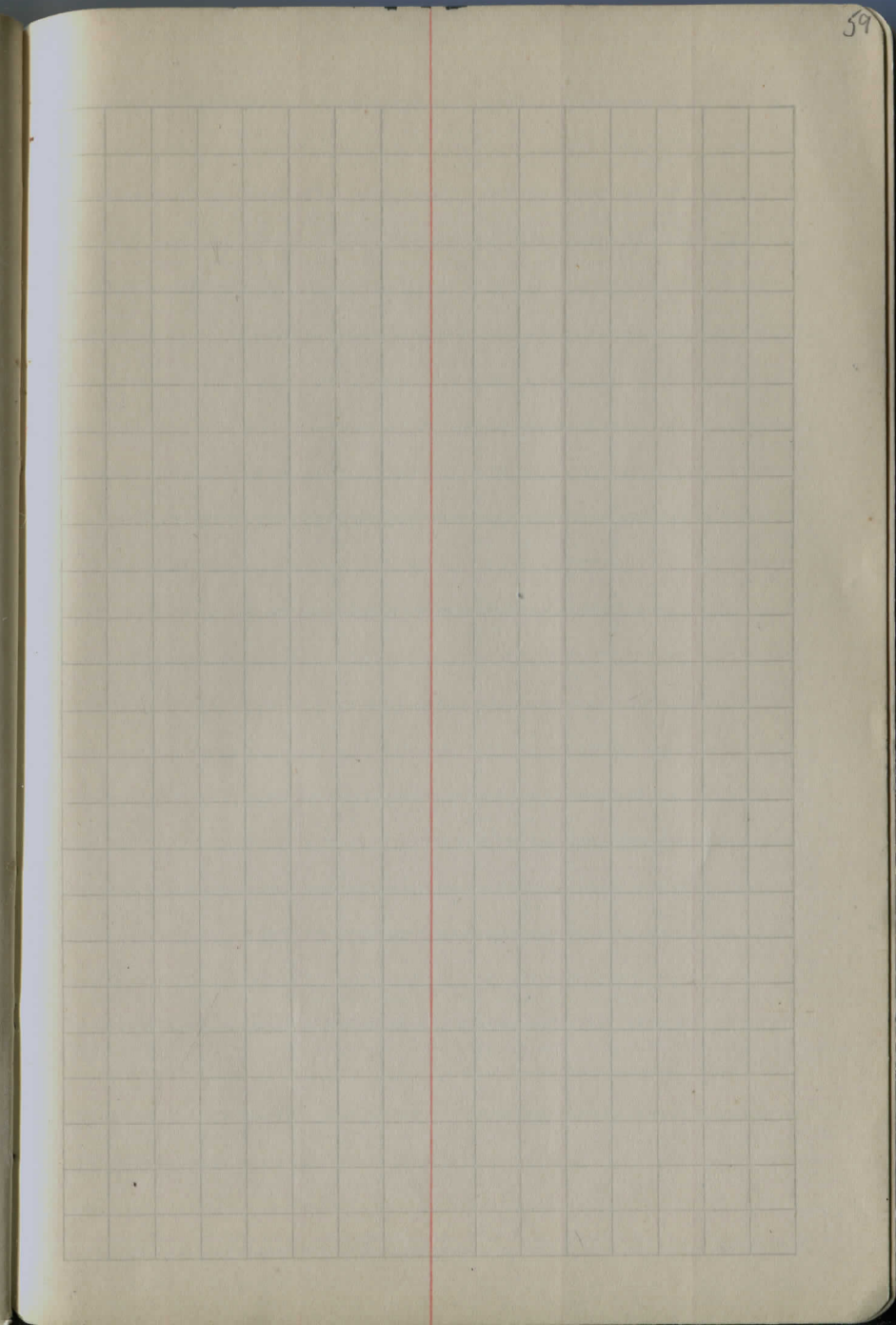
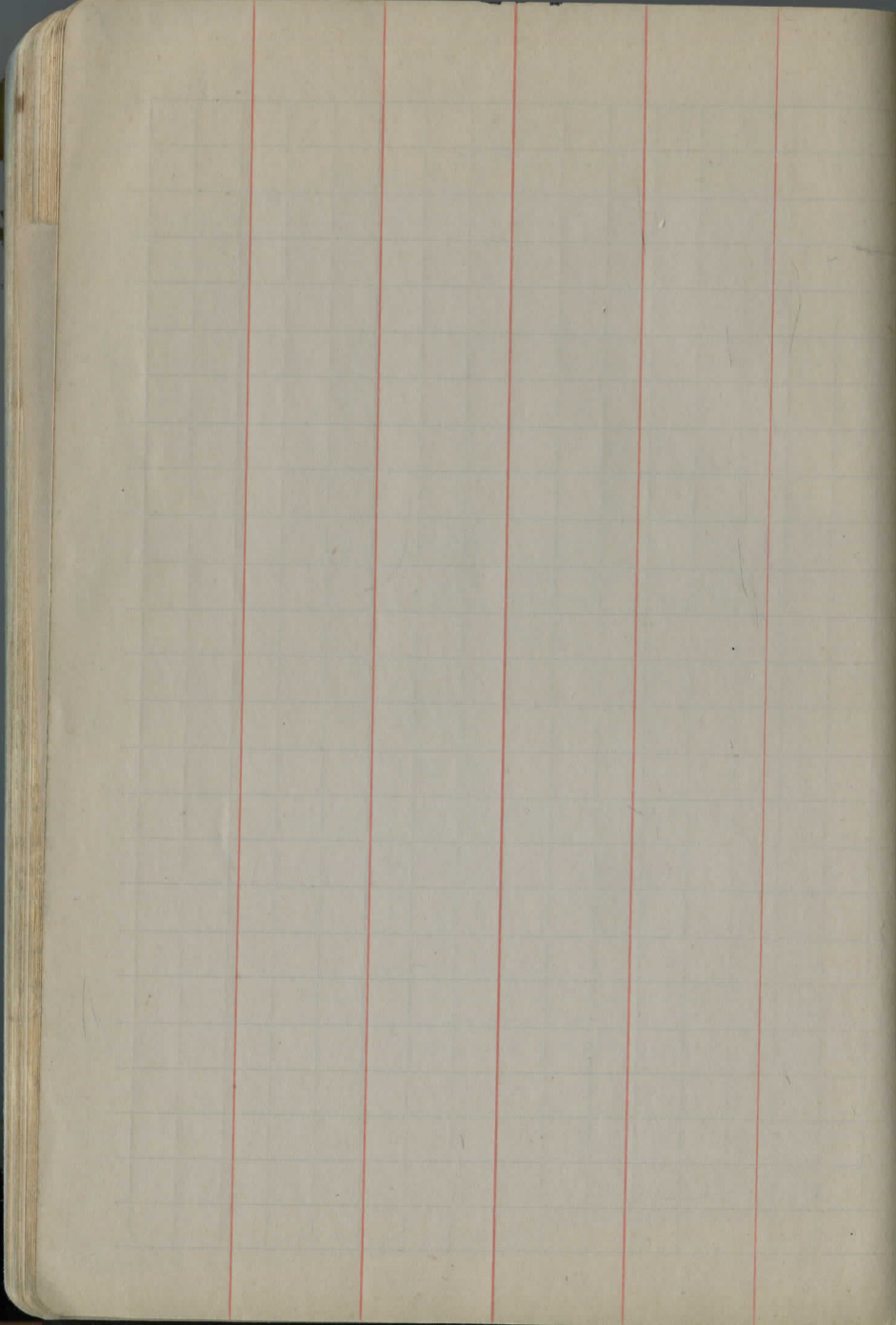
$$\begin{array}{r} 35 \\ \underline{35} \\ 175 \\ \underline{105} \\ 1225 \\ \underline{7} \end{array}$$

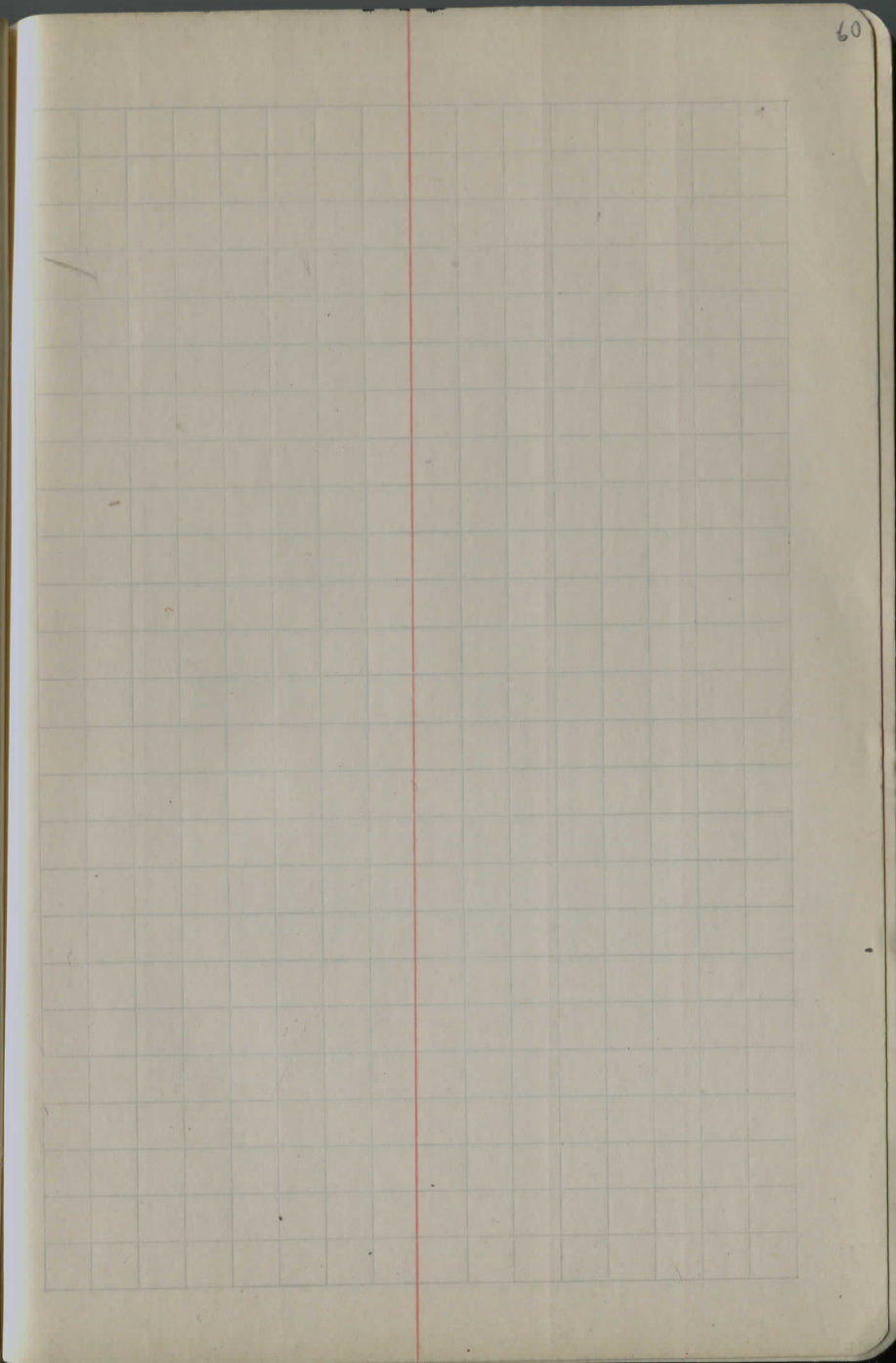
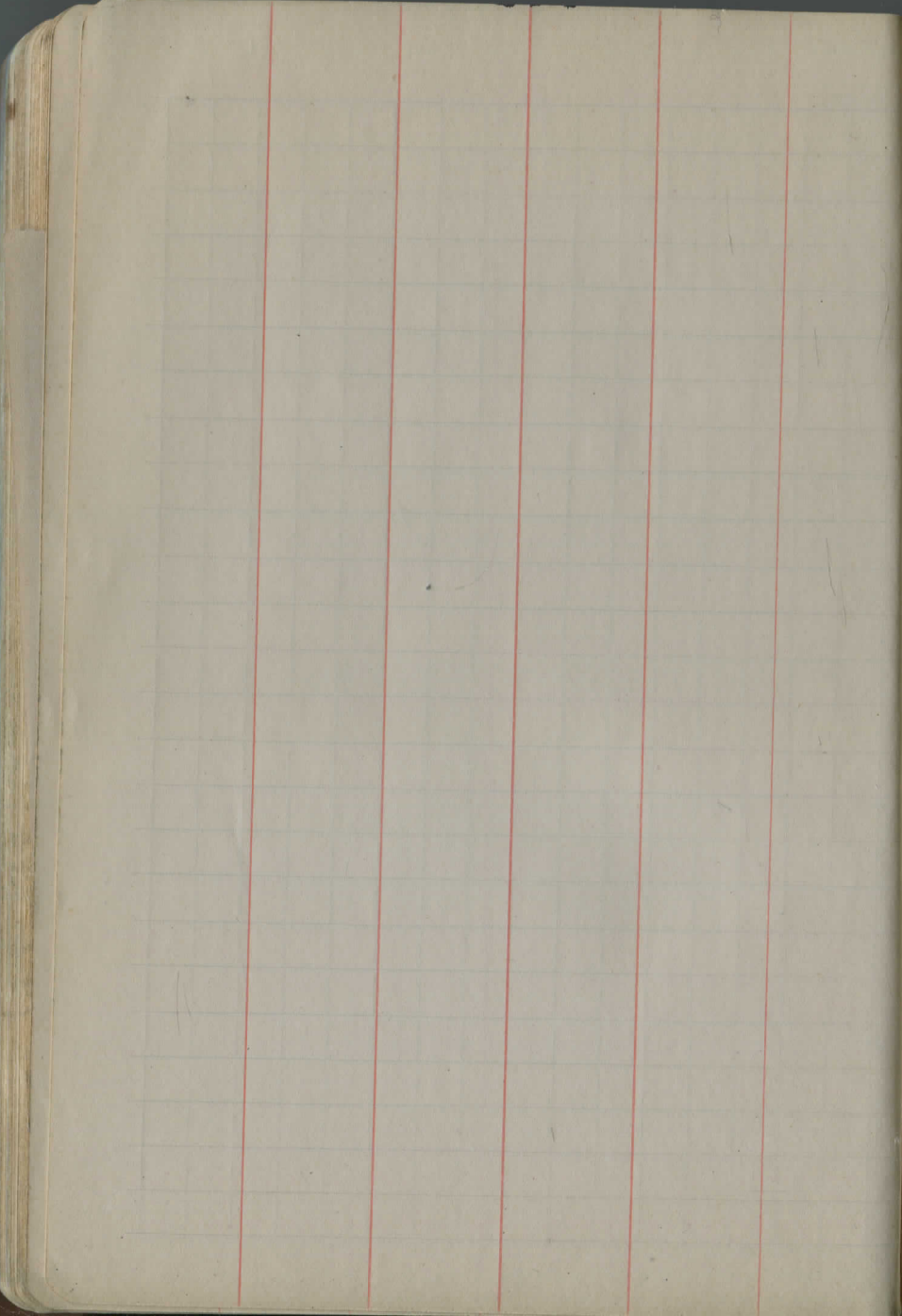
$$\frac{7}{8} \times 4 = 28$$

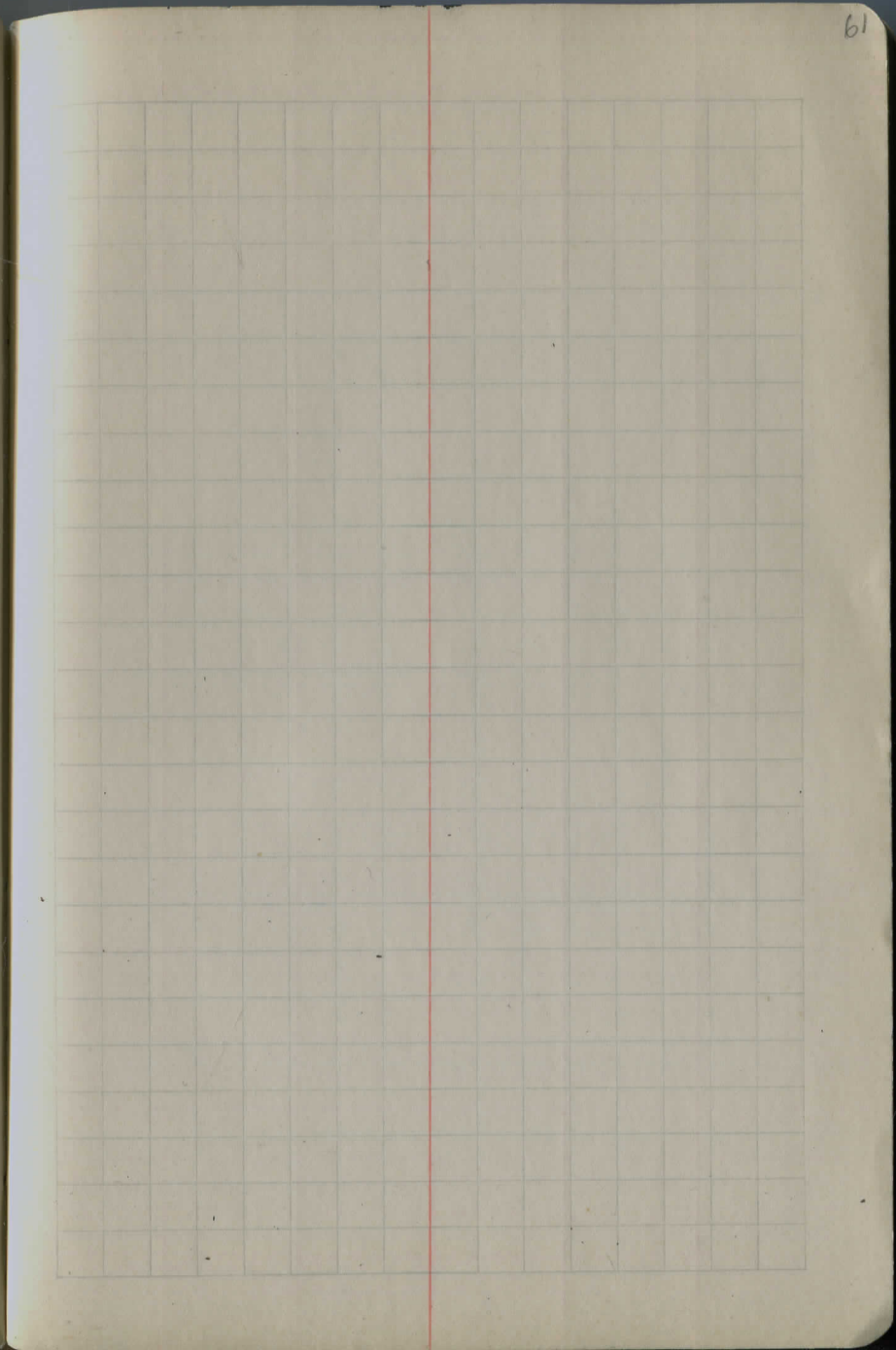
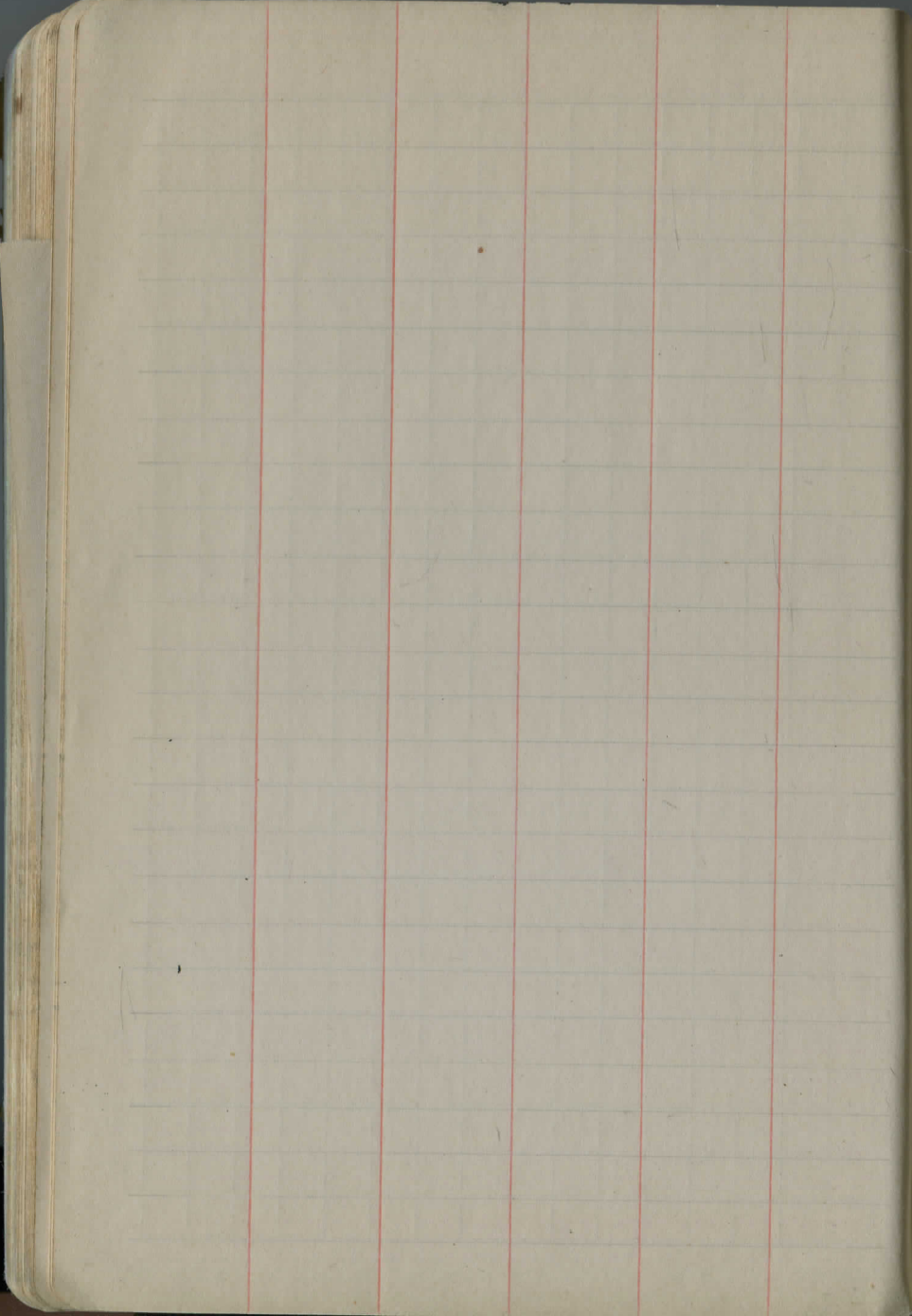
$$\frac{7}{8} \times 2.14 = \frac{14}{4} = 3.5$$

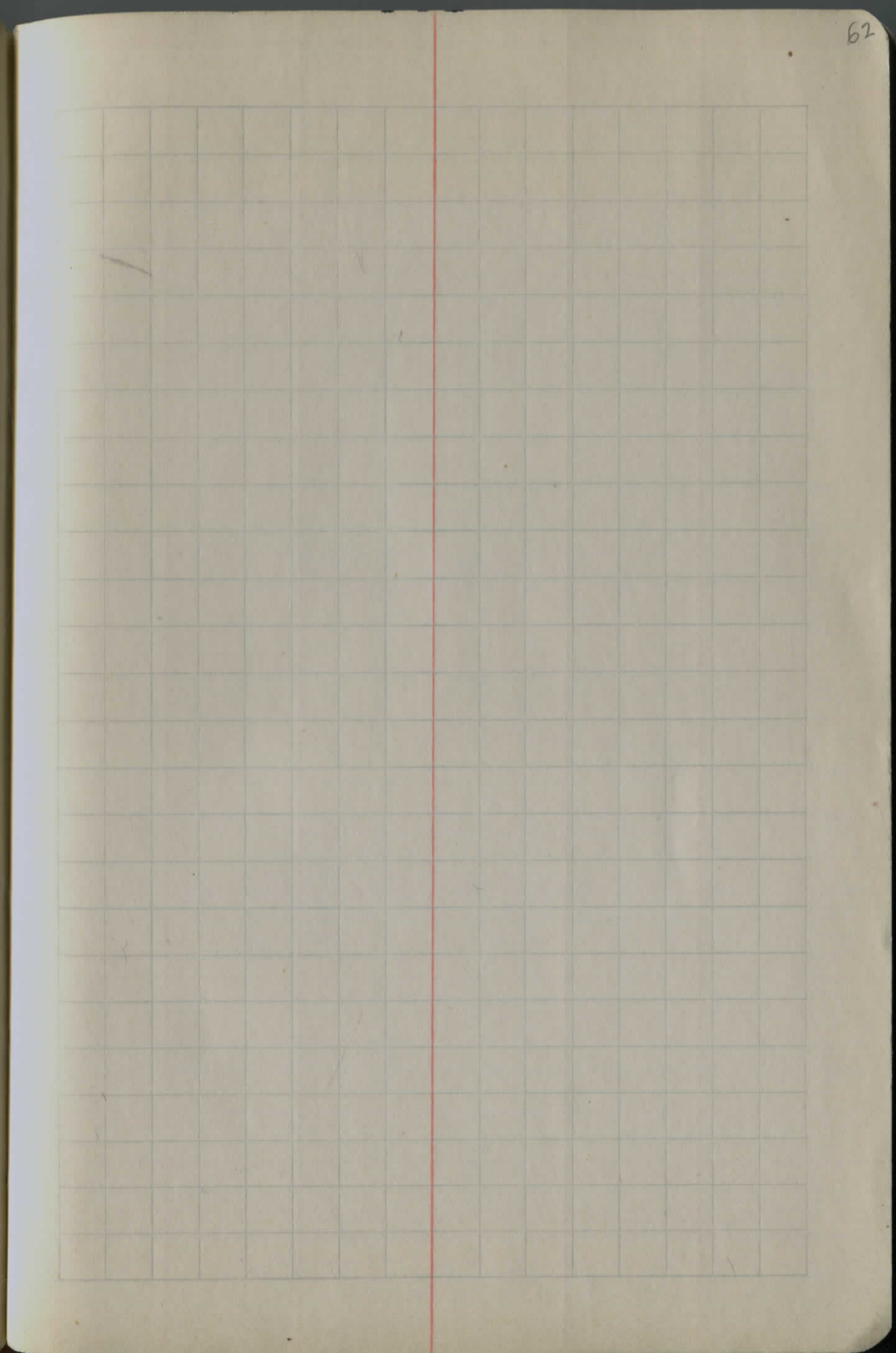
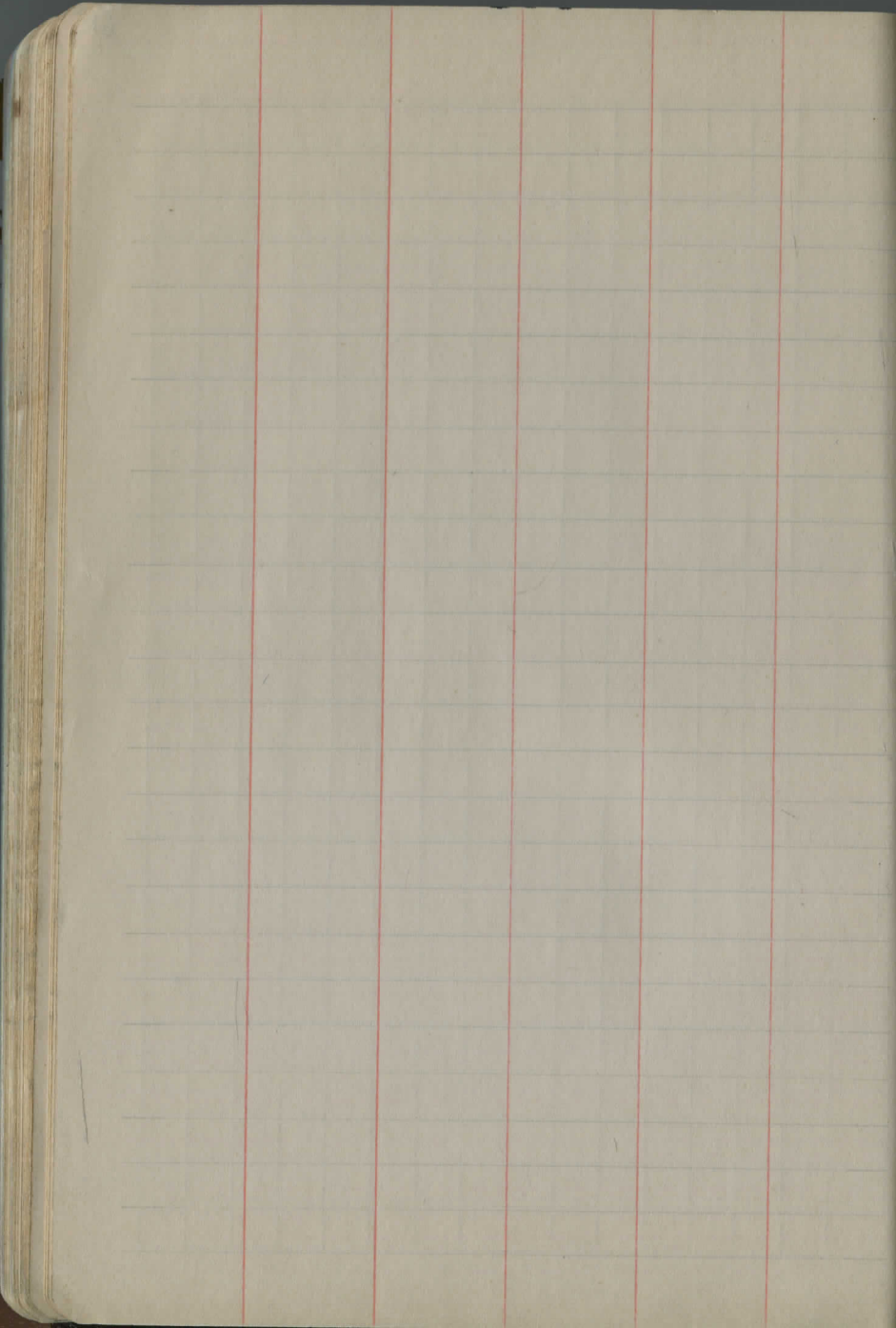
$$36.82$$

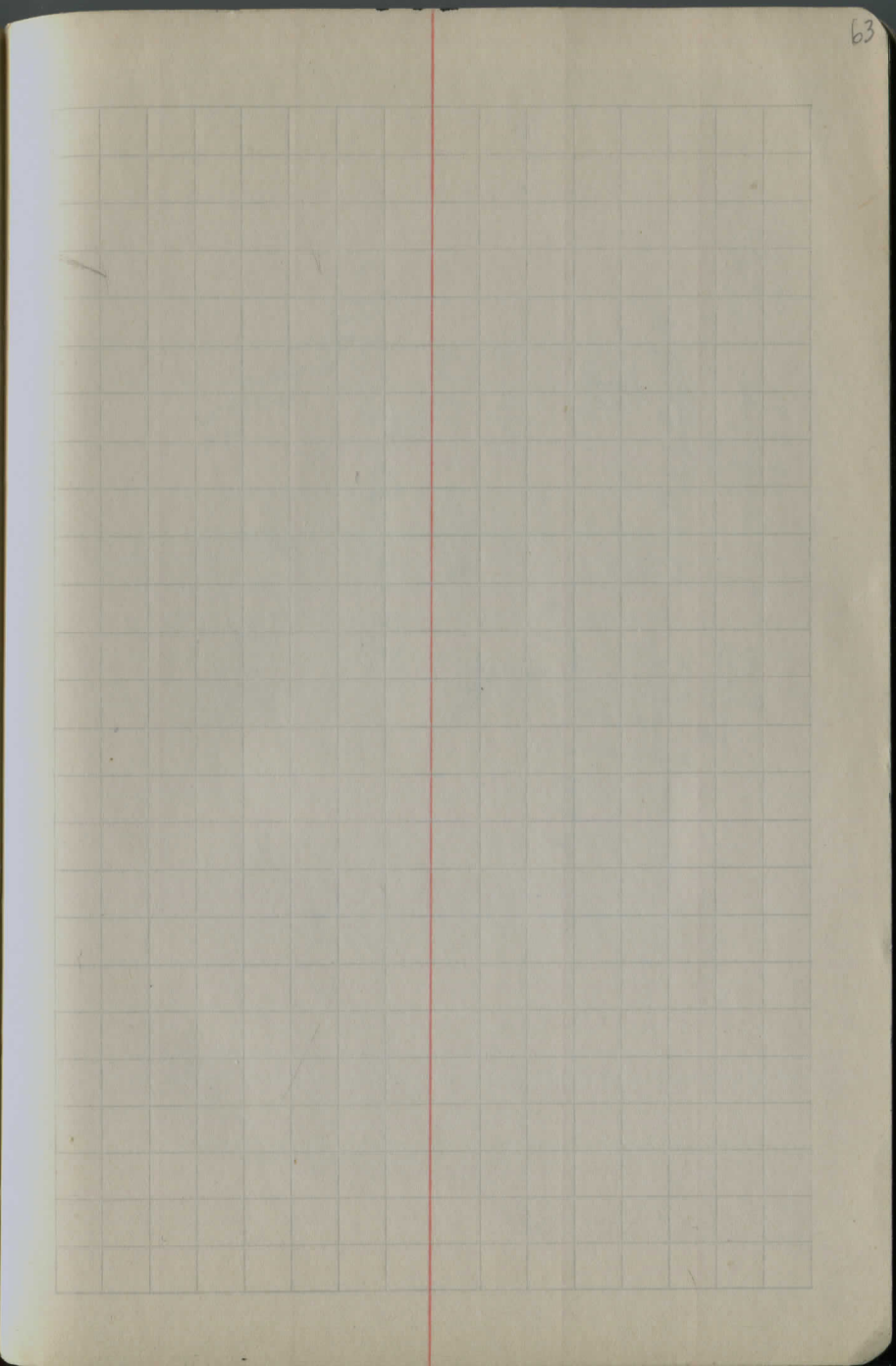
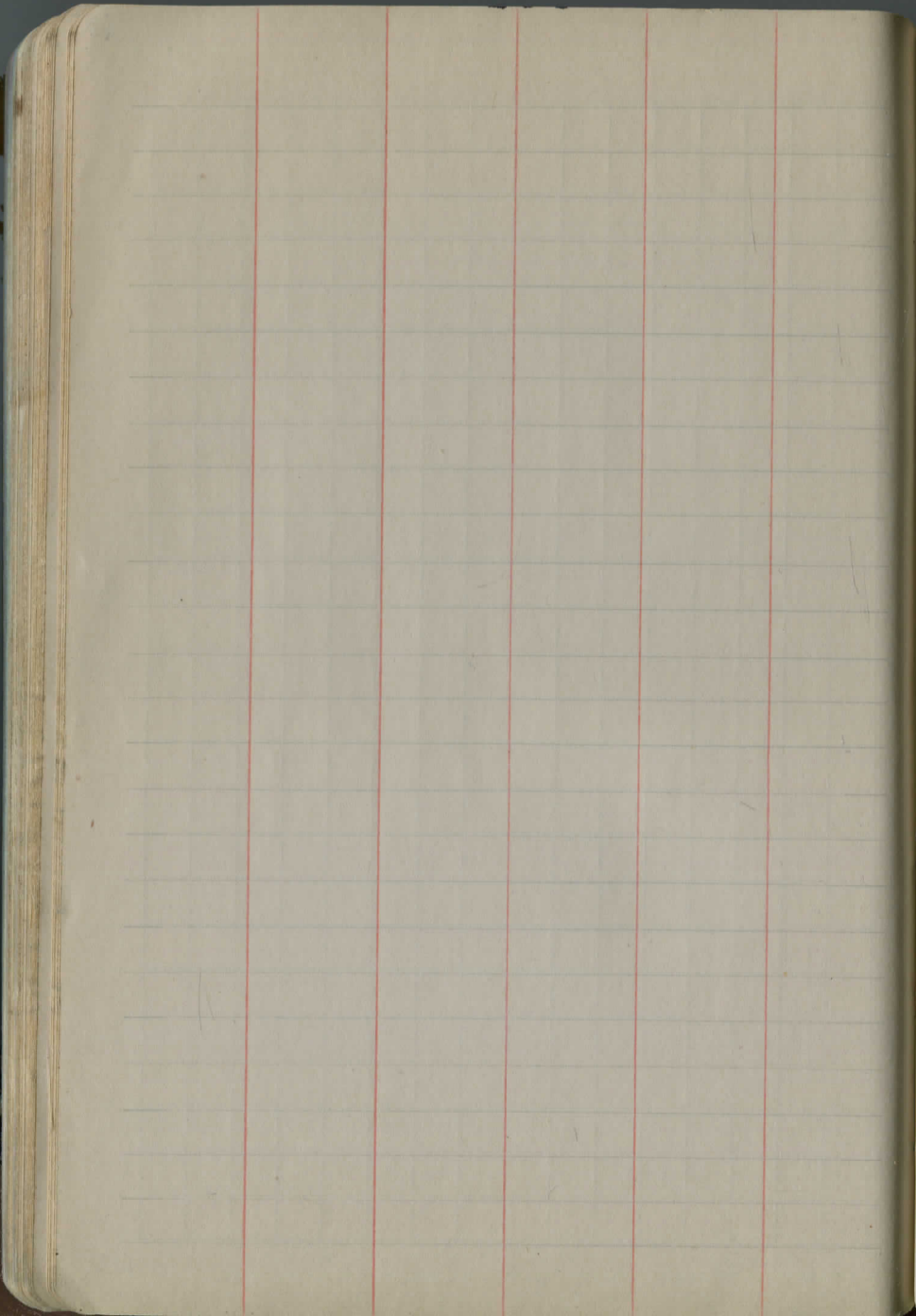
$$\begin{array}{r} 7+31.15 \\ \underline{68.85} \\ 36.82 \\ \underline{104.27} \end{array}$$

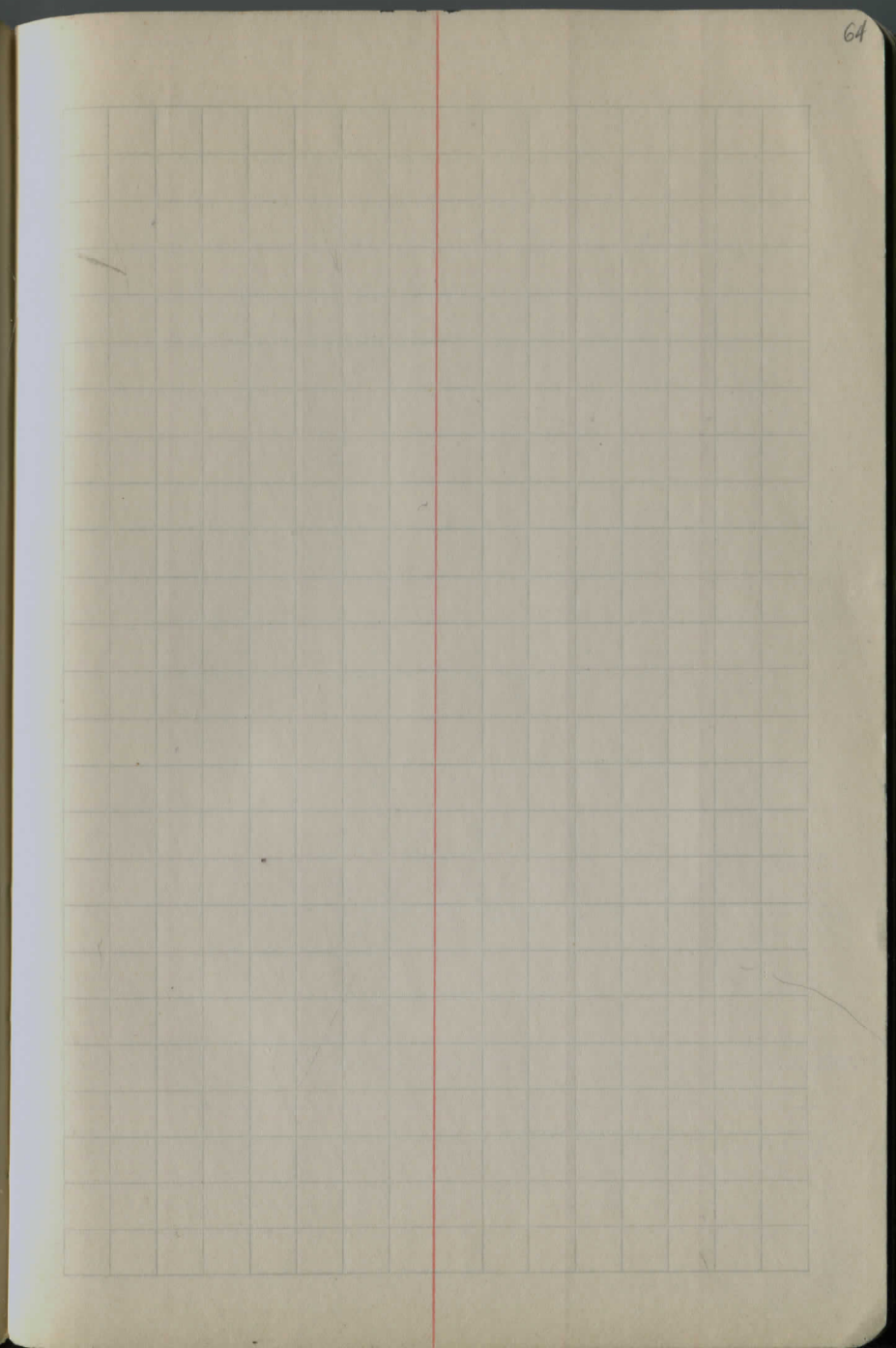
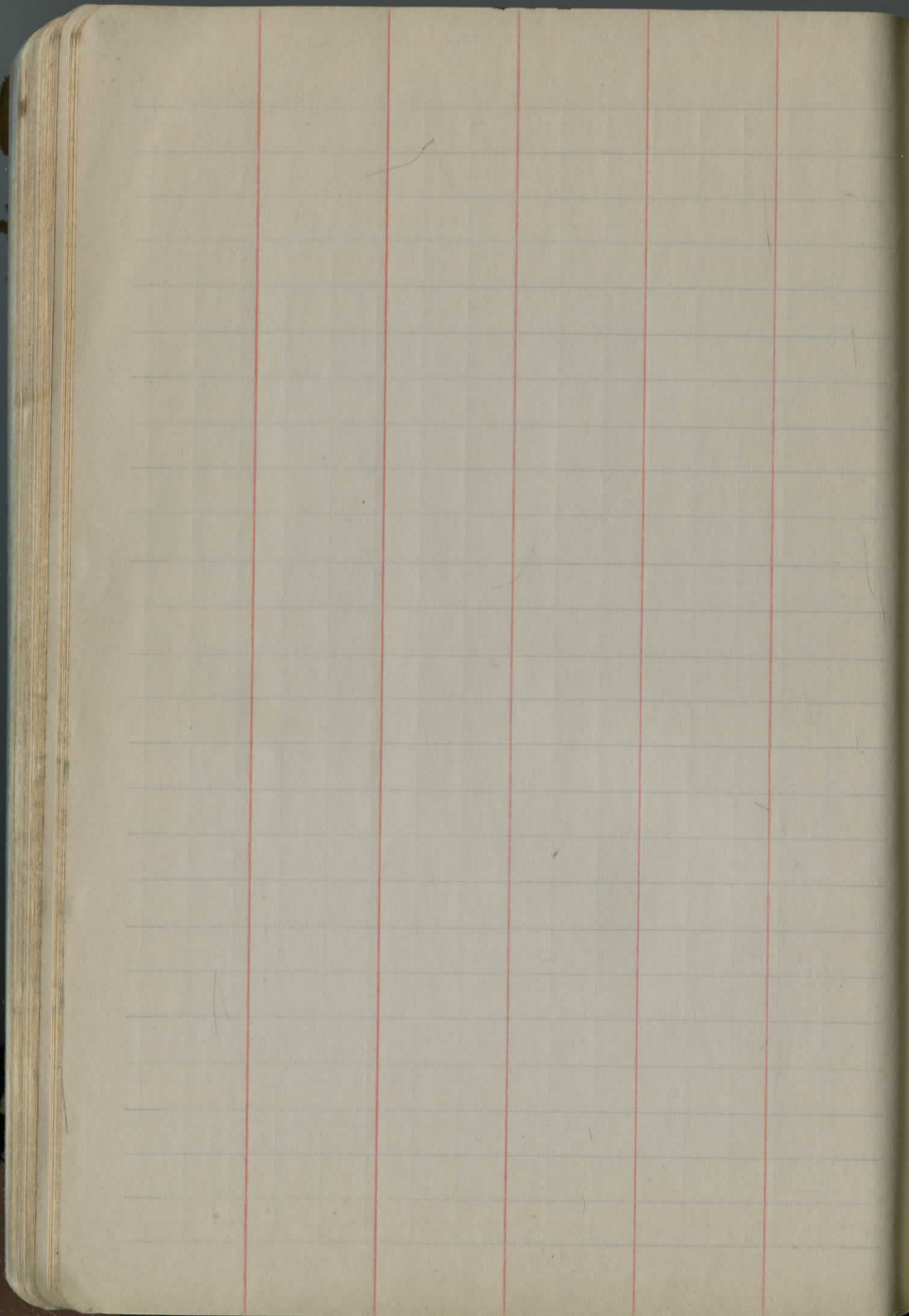












$$\begin{array}{r} 179-60 \\ 22-05 \\ \hline 157-55 \end{array}$$

$$\begin{array}{r} 108-90 \\ 107-24 \\ \hline 1-66 \\ 83 \end{array}$$

$$\begin{array}{r} 1113-7 \\ 1122-4 \\ \hline 13 \end{array}$$

$$12) 1007.24$$

$$\begin{array}{r} 84 \\ \underline{48} \\ 48 \\ \underline{48} \\ 01 \end{array}$$

$$\begin{array}{r} 1122-4 \\ 1113-2 \\ \hline 4-7 \\ 2.3 \end{array}$$

$$6) 1113.7$$

$$\begin{array}{r} 186 \\ \underline{6} \\ 57 \\ \underline{48} \\ 36 \end{array}$$

372

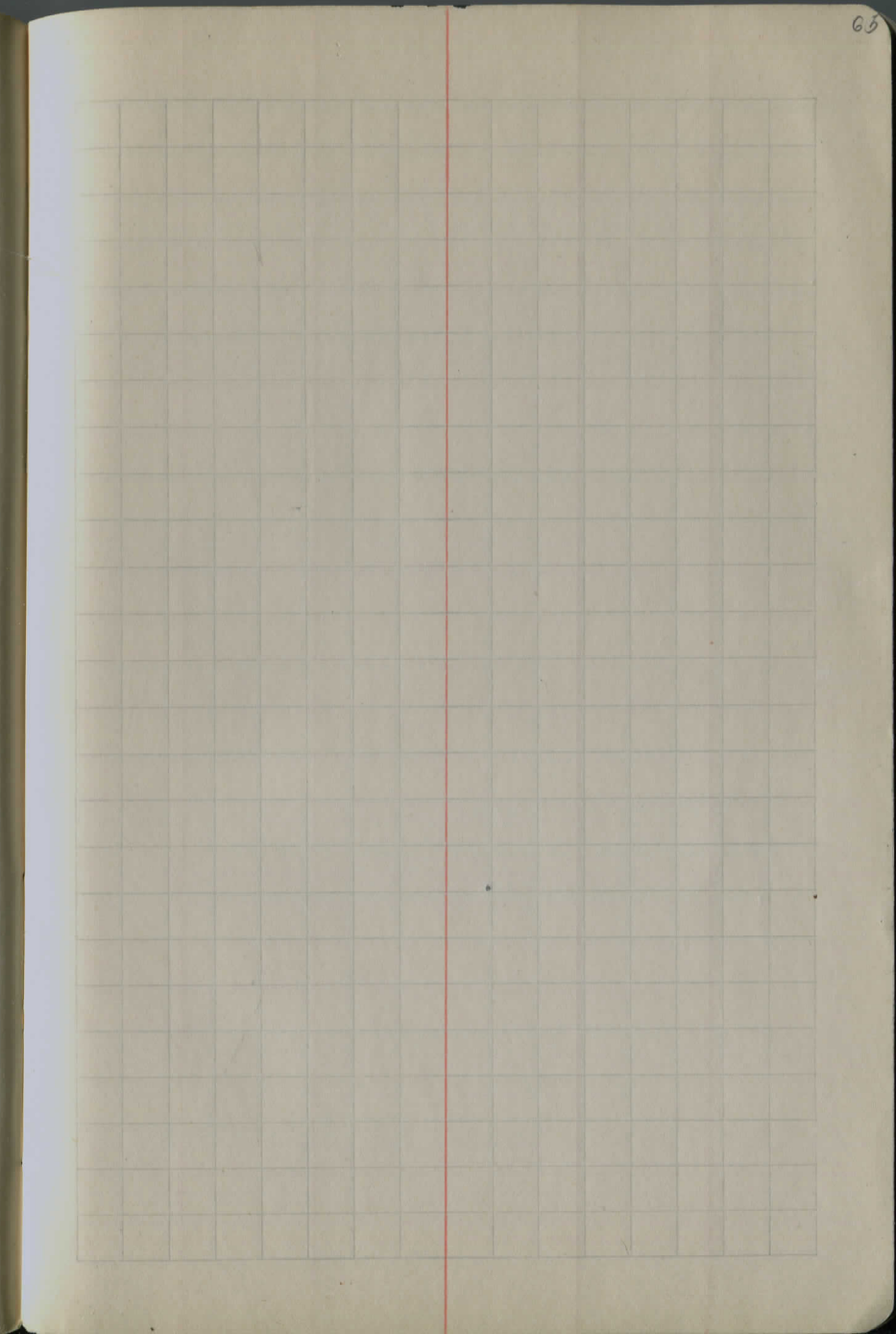
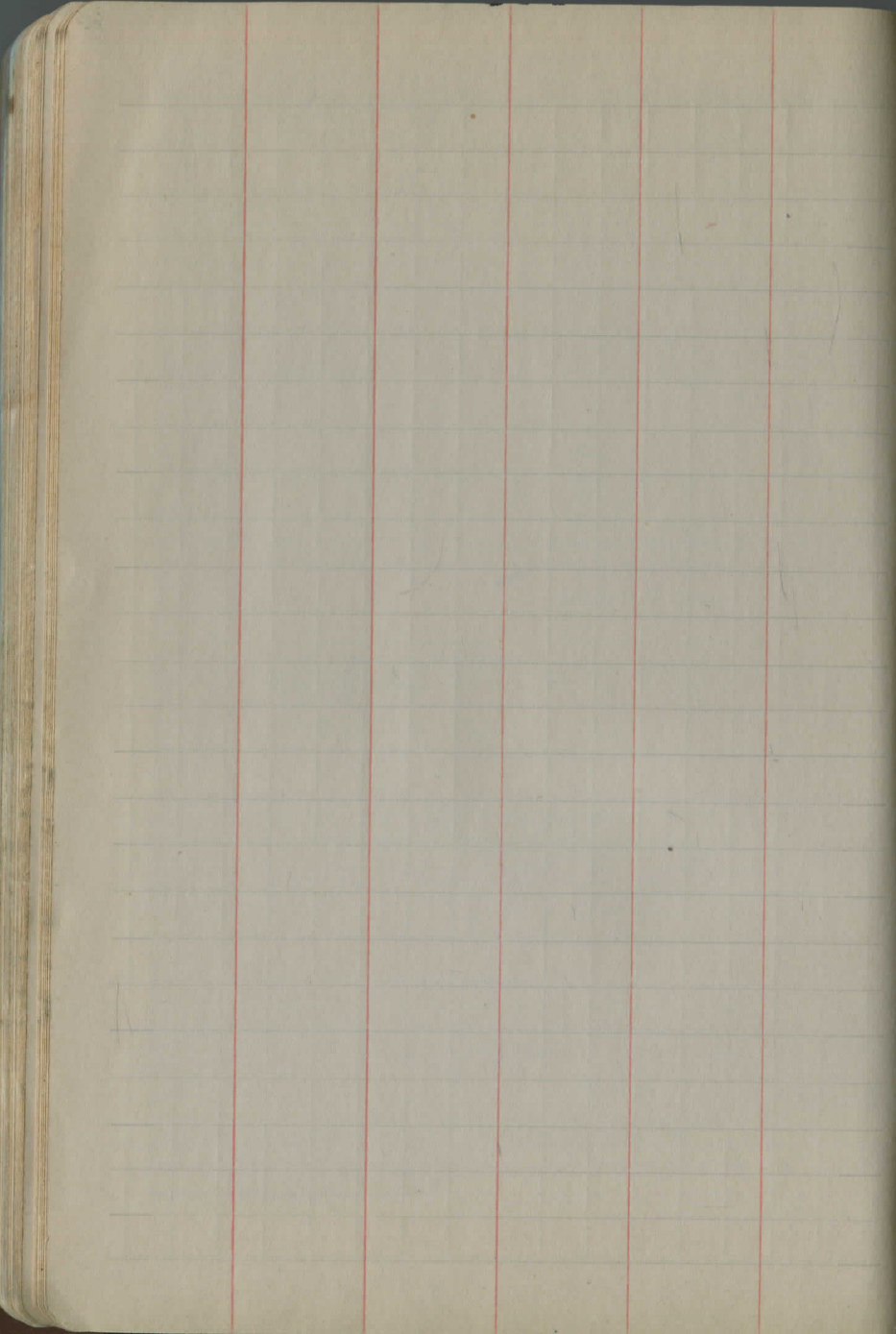
$$6) 5000$$

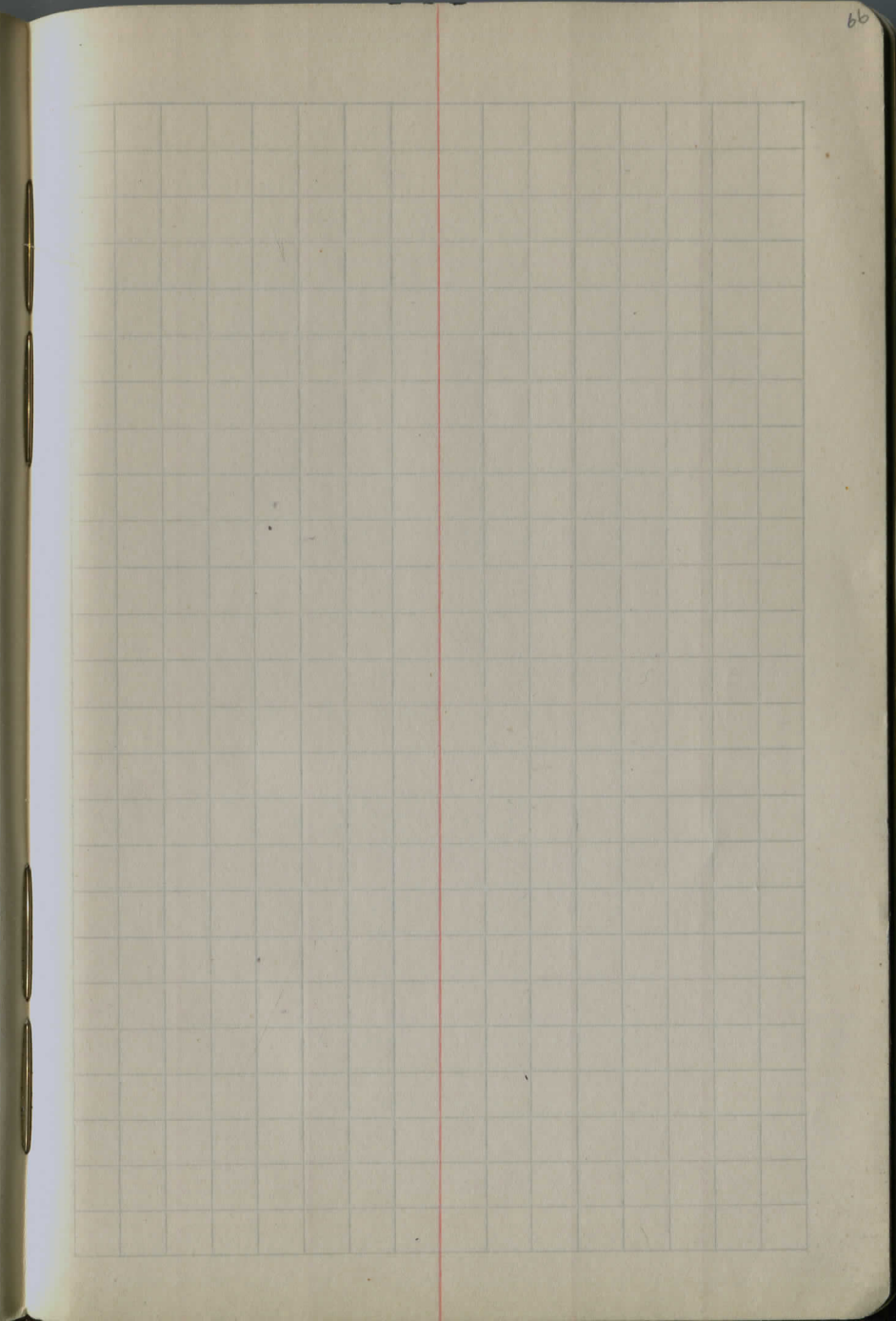
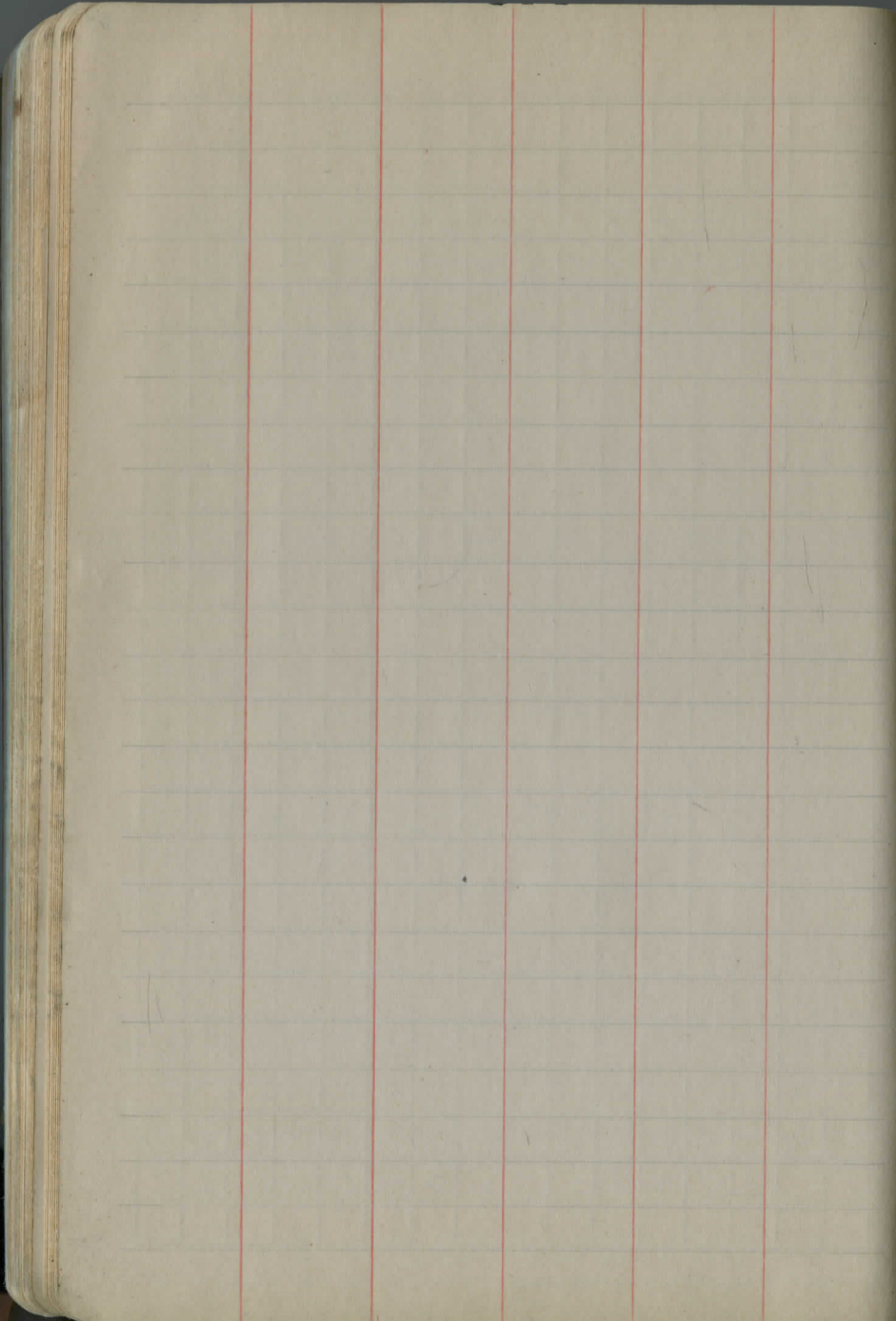
$$\begin{array}{r} 833 \\ \underline{48} \\ 20 \\ \underline{18} \\ 20 \end{array}$$

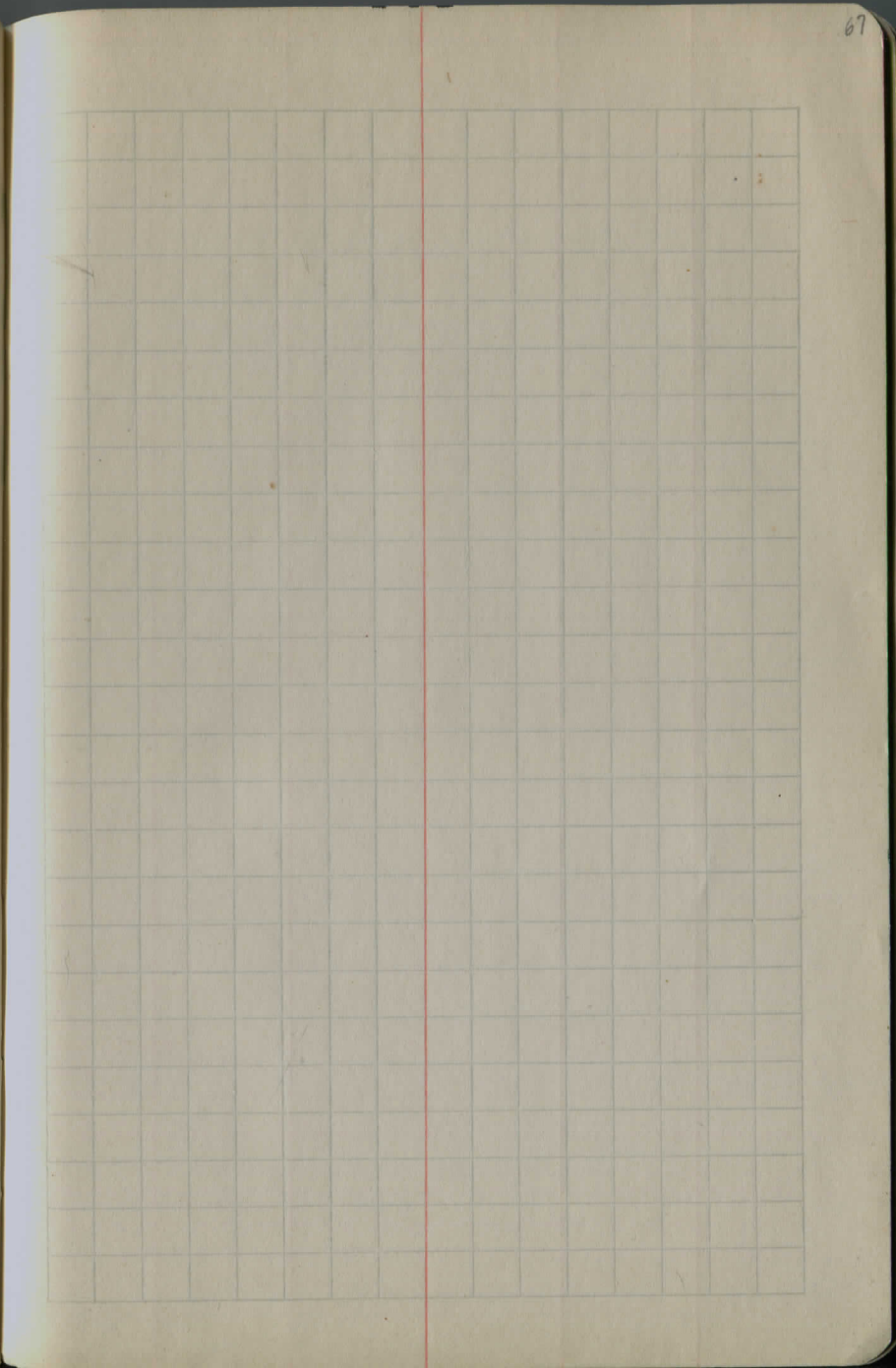
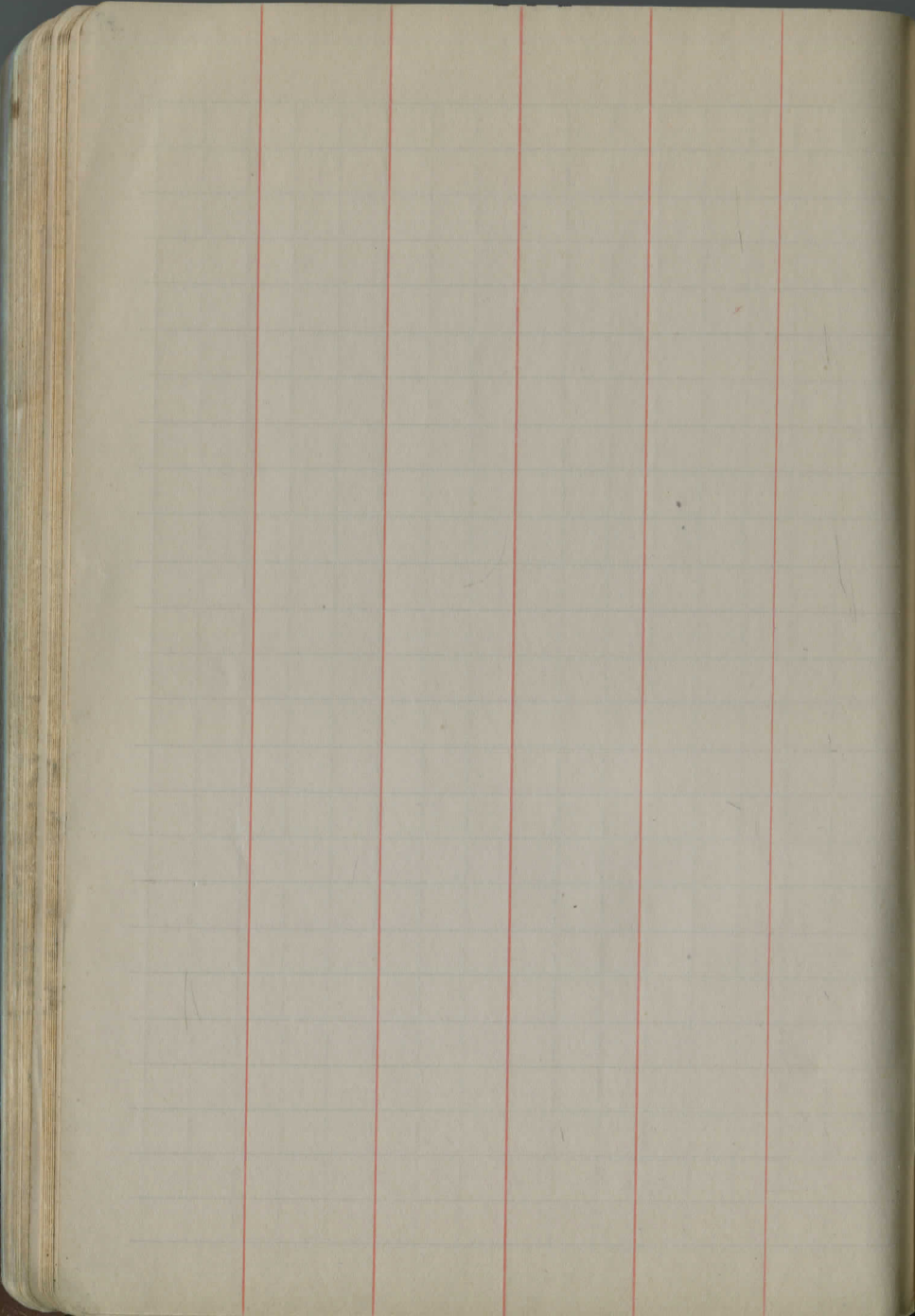
$$6) 22.08$$

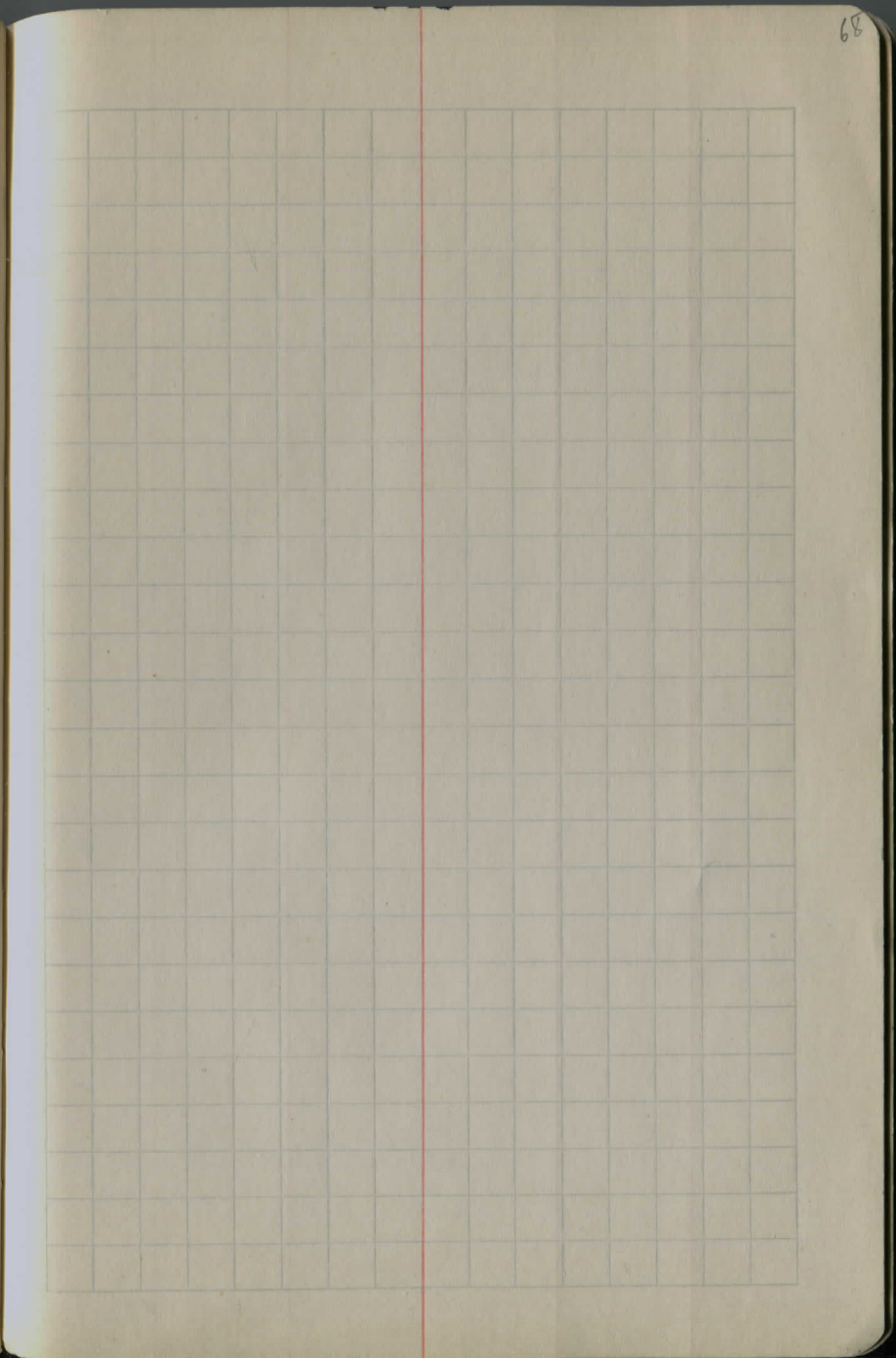
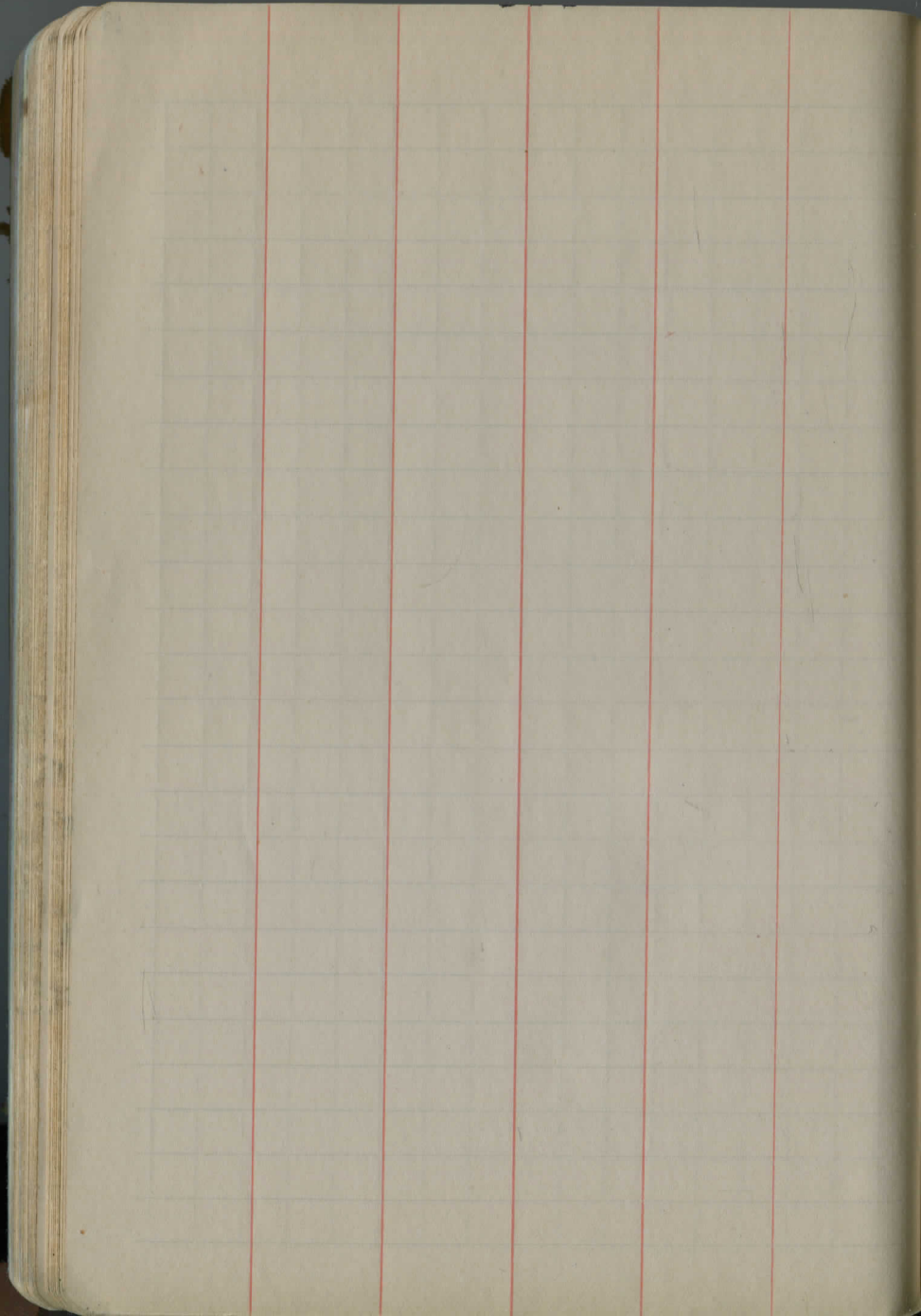
$$\begin{array}{r} 36 \\ \underline{18} \\ 40 \\ \underline{36} \\ 28 \\ \underline{24} \\ 42 \\ \underline{42} \\ 0 \end{array}$$

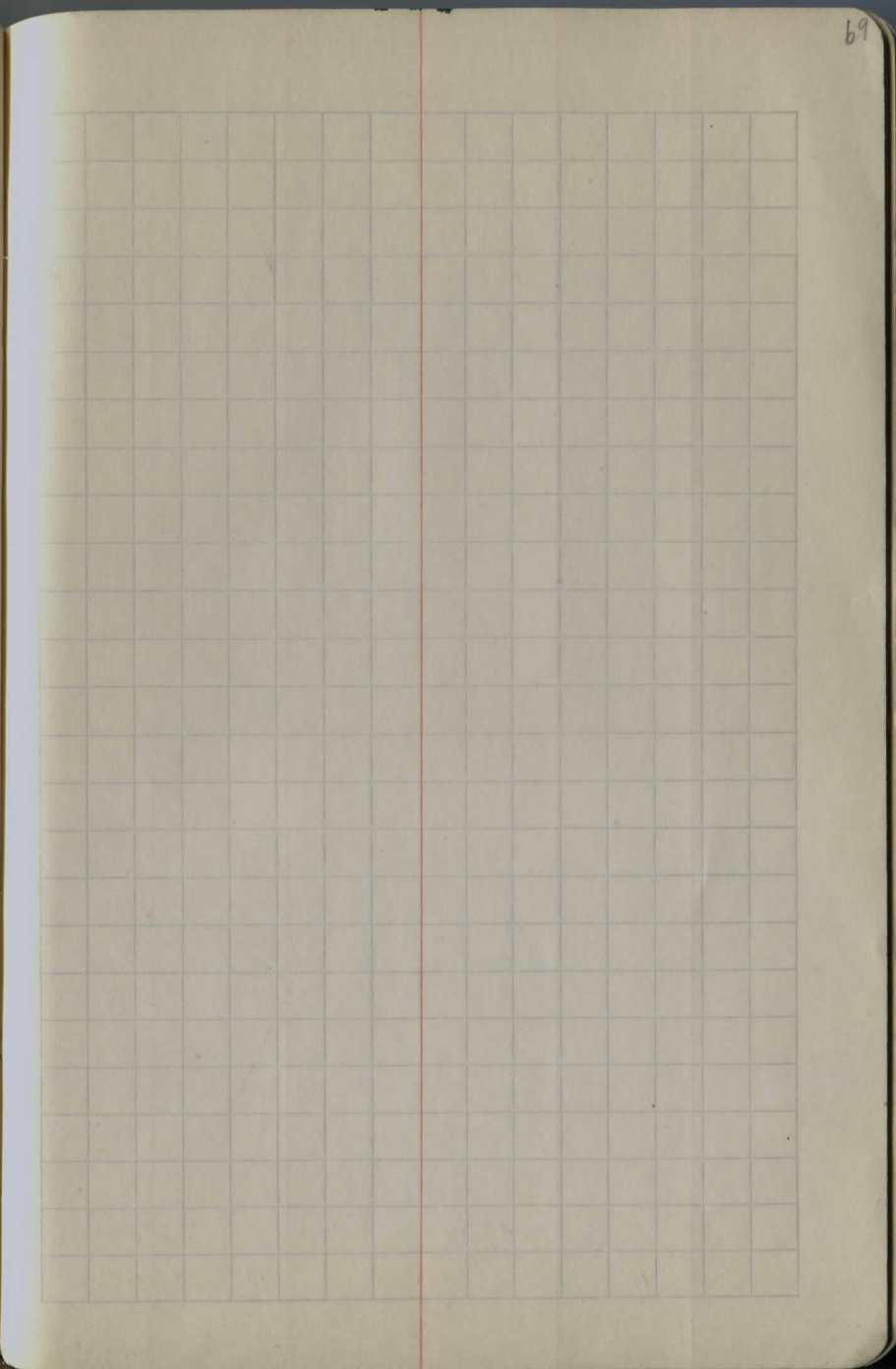
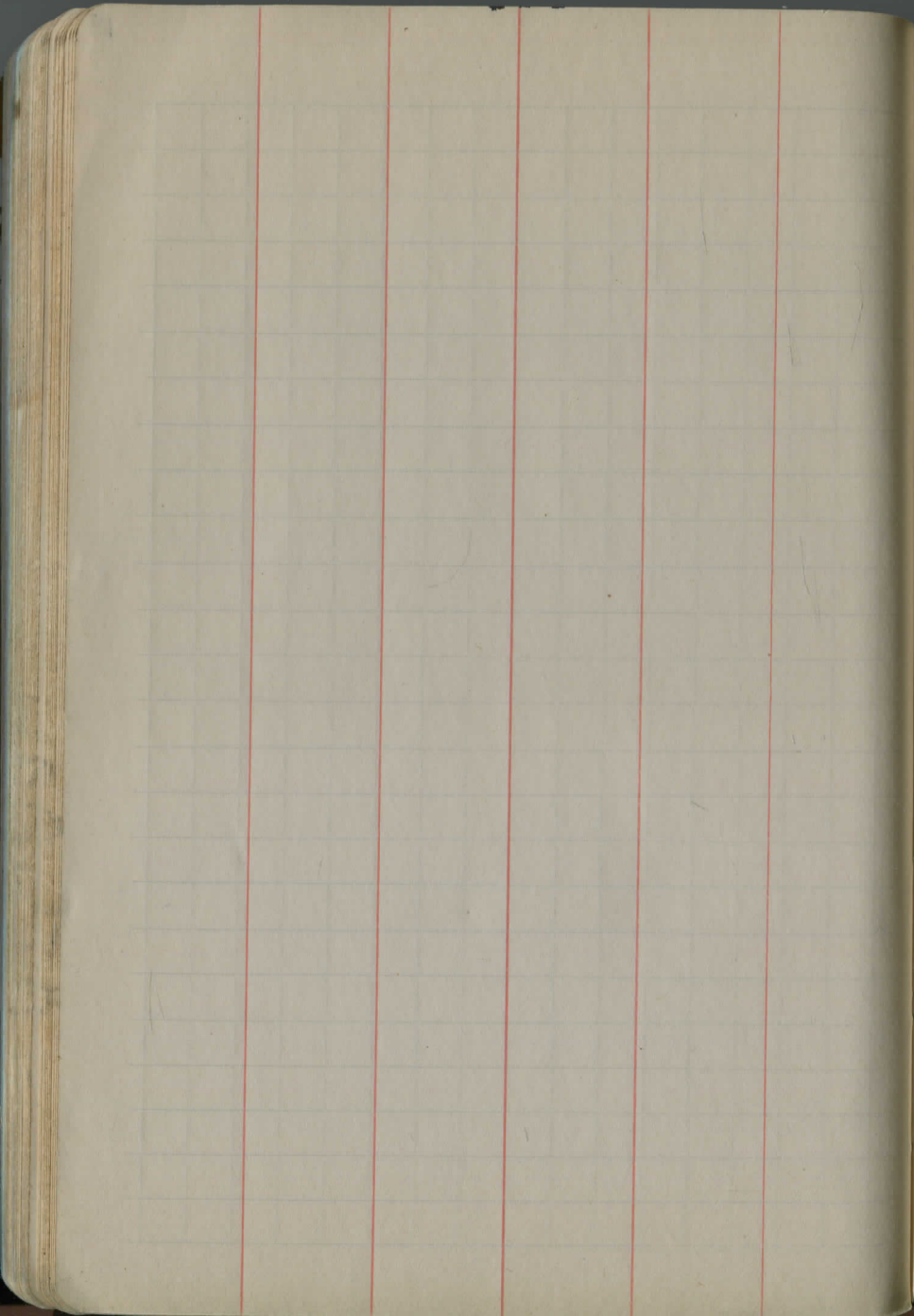
$$\frac{2}{8} 76 \div 42.5$$

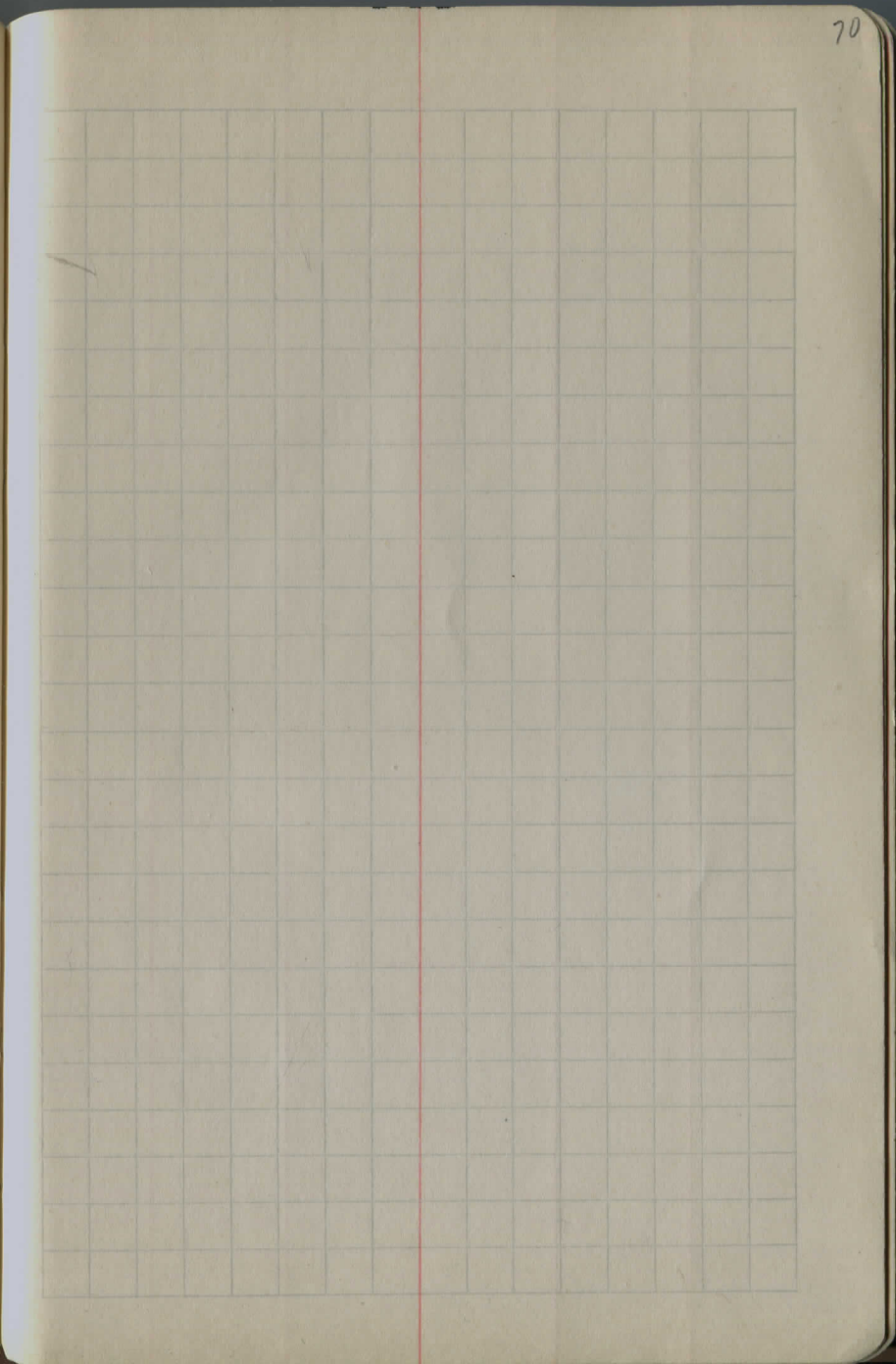
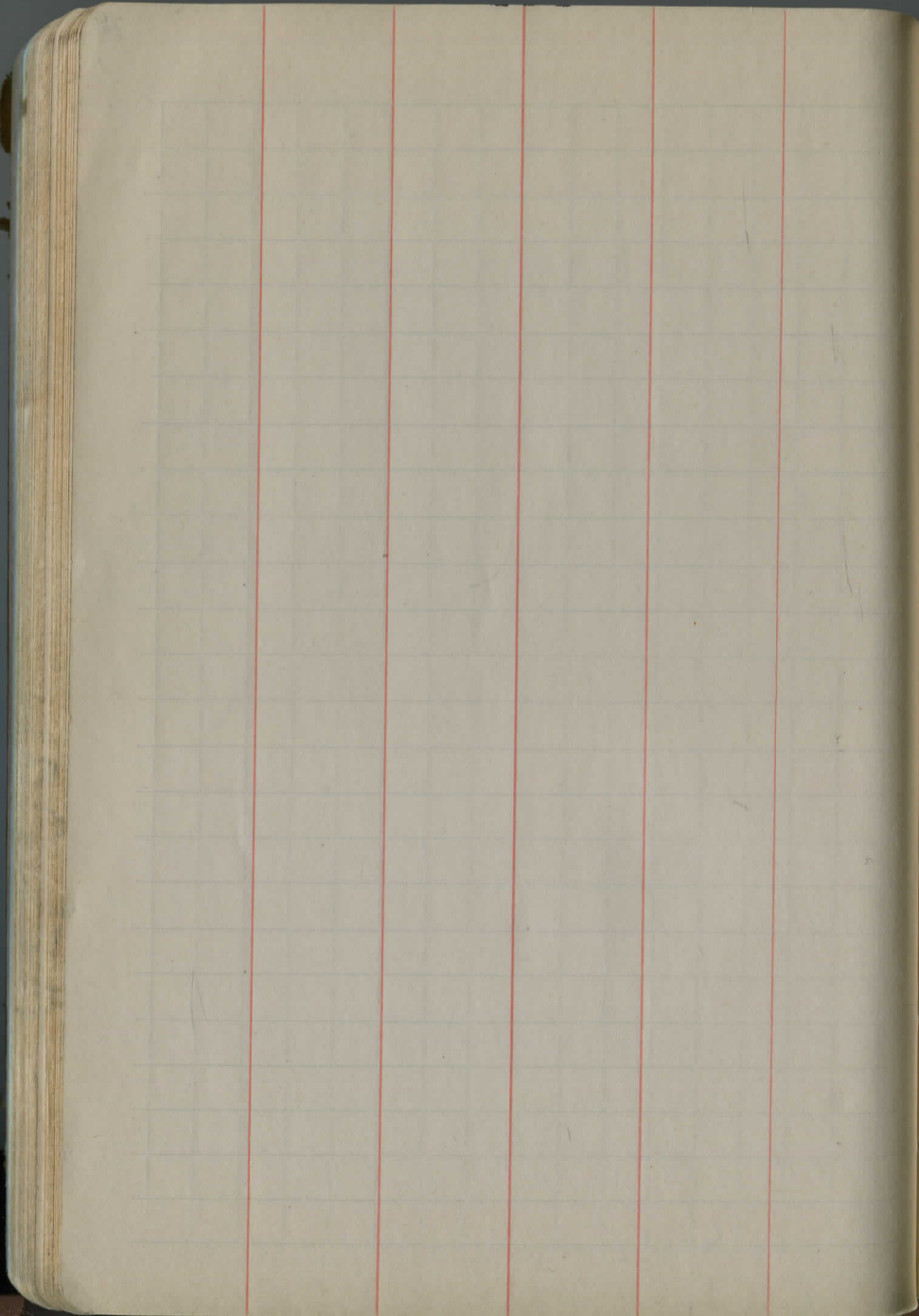


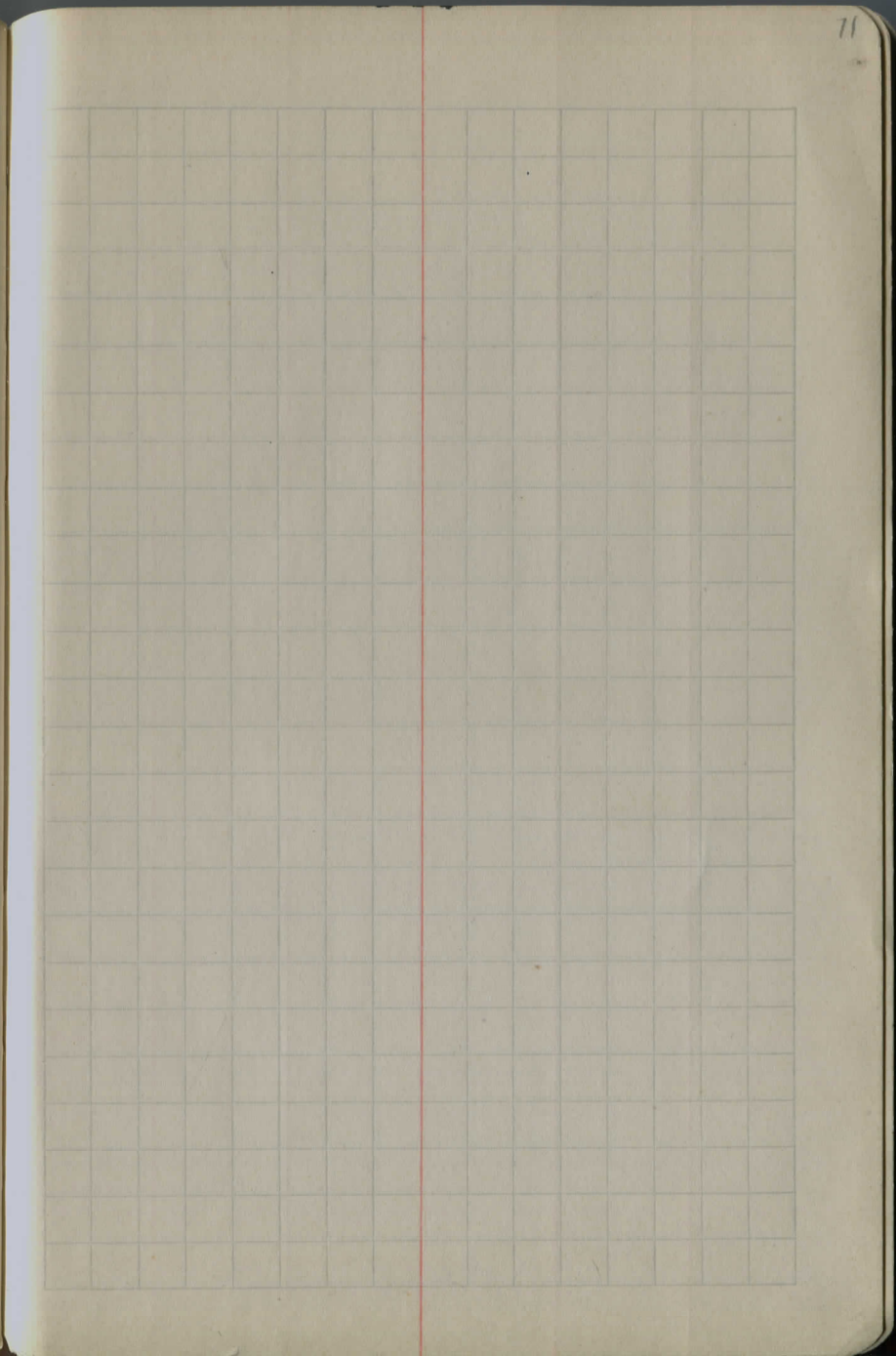
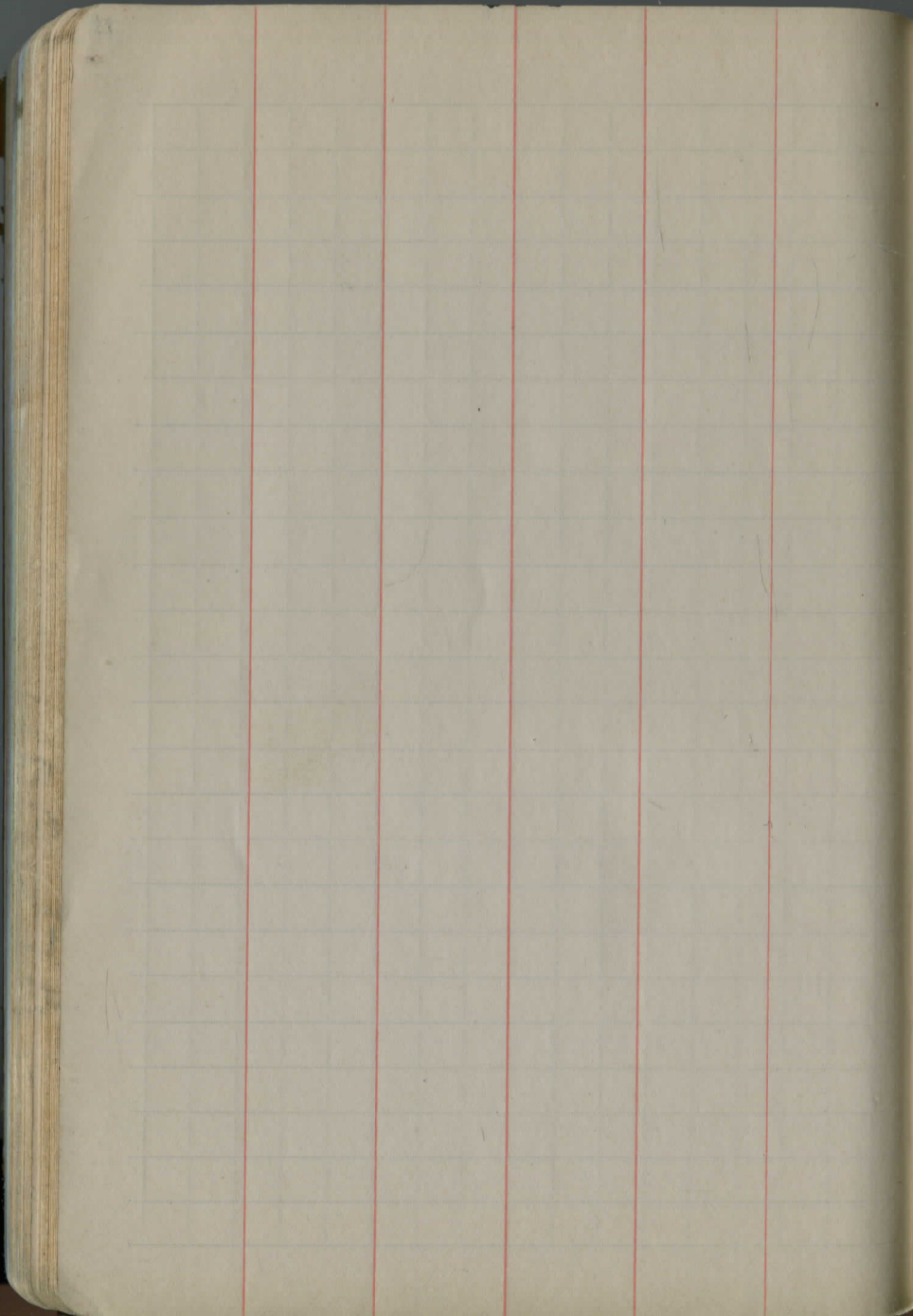


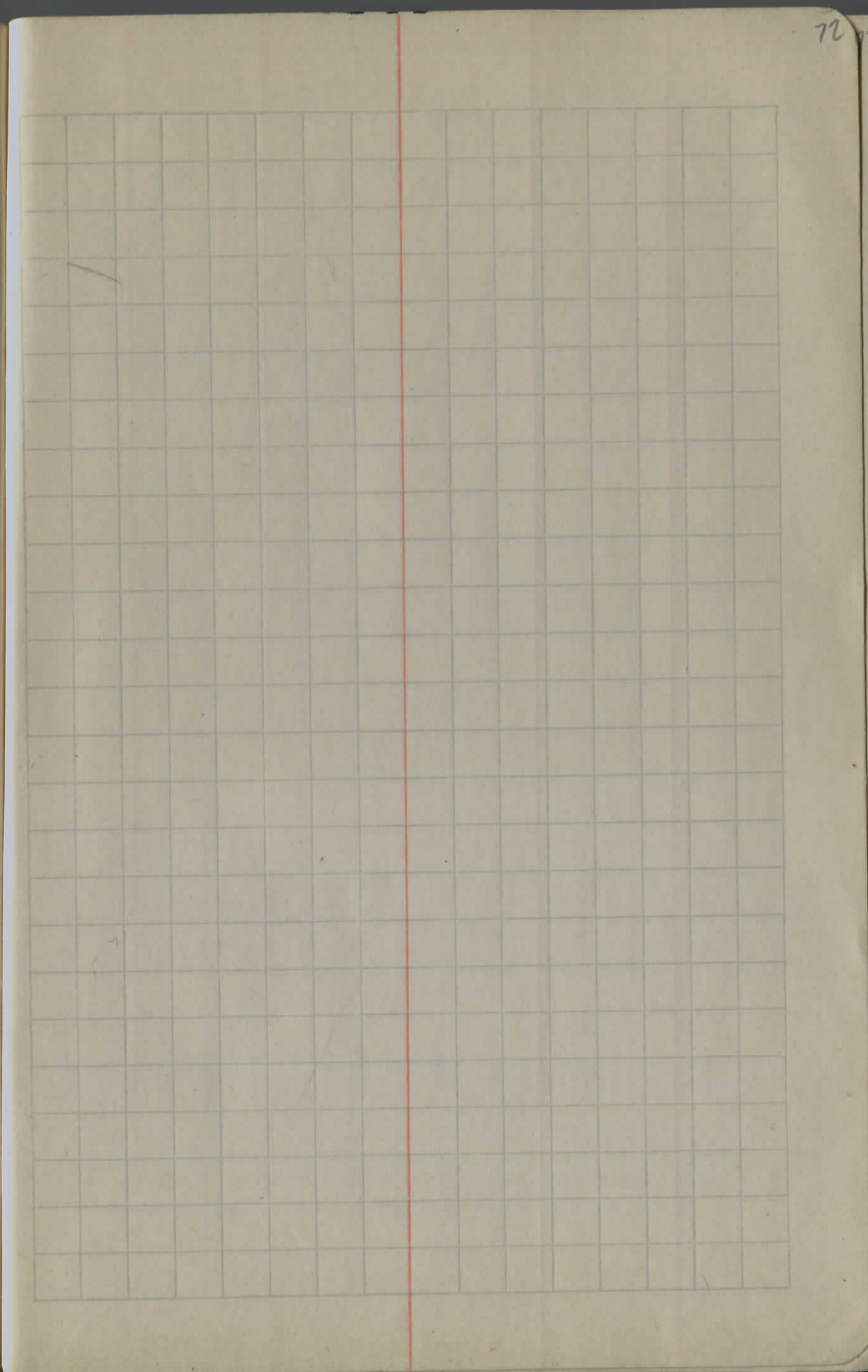
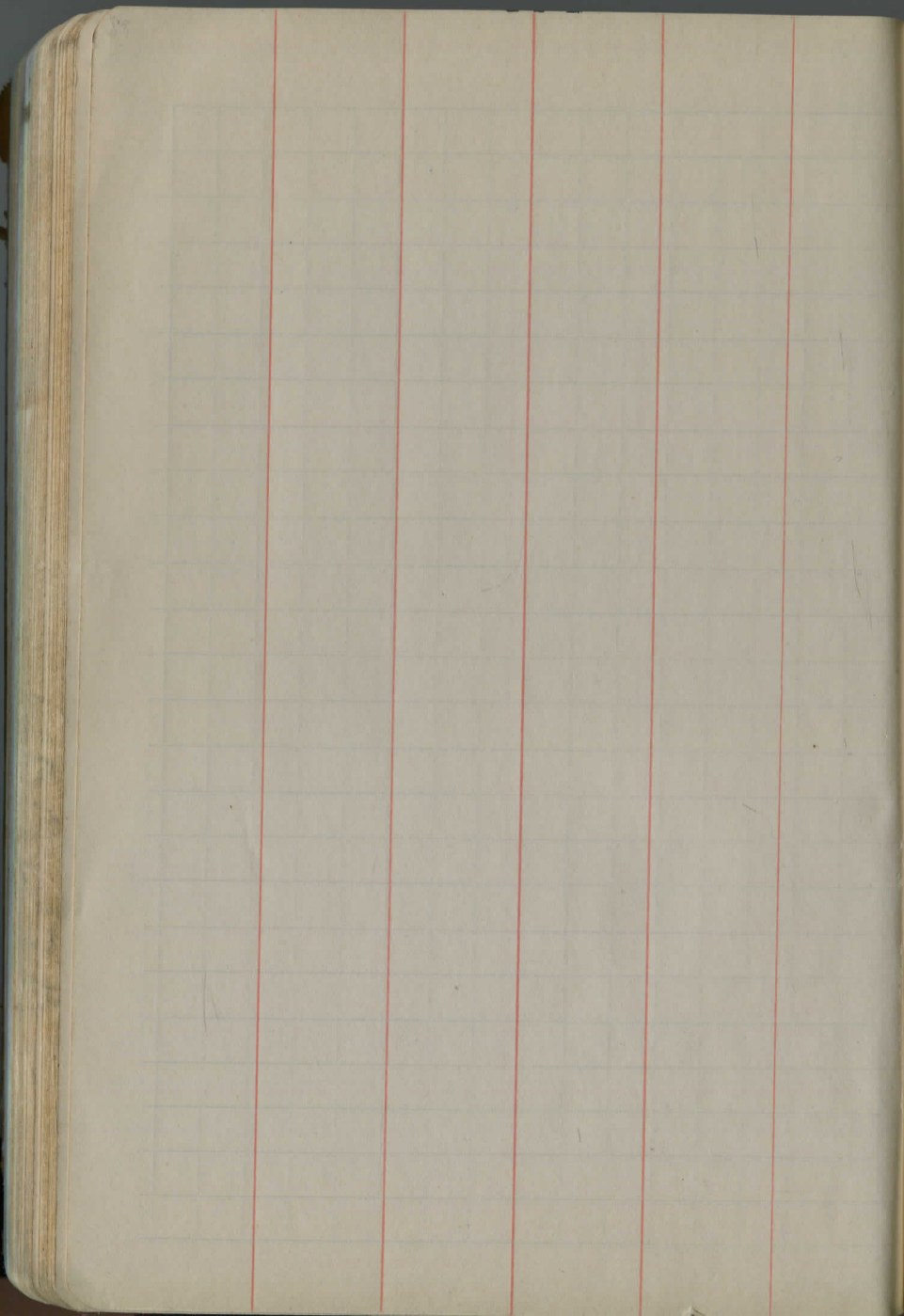


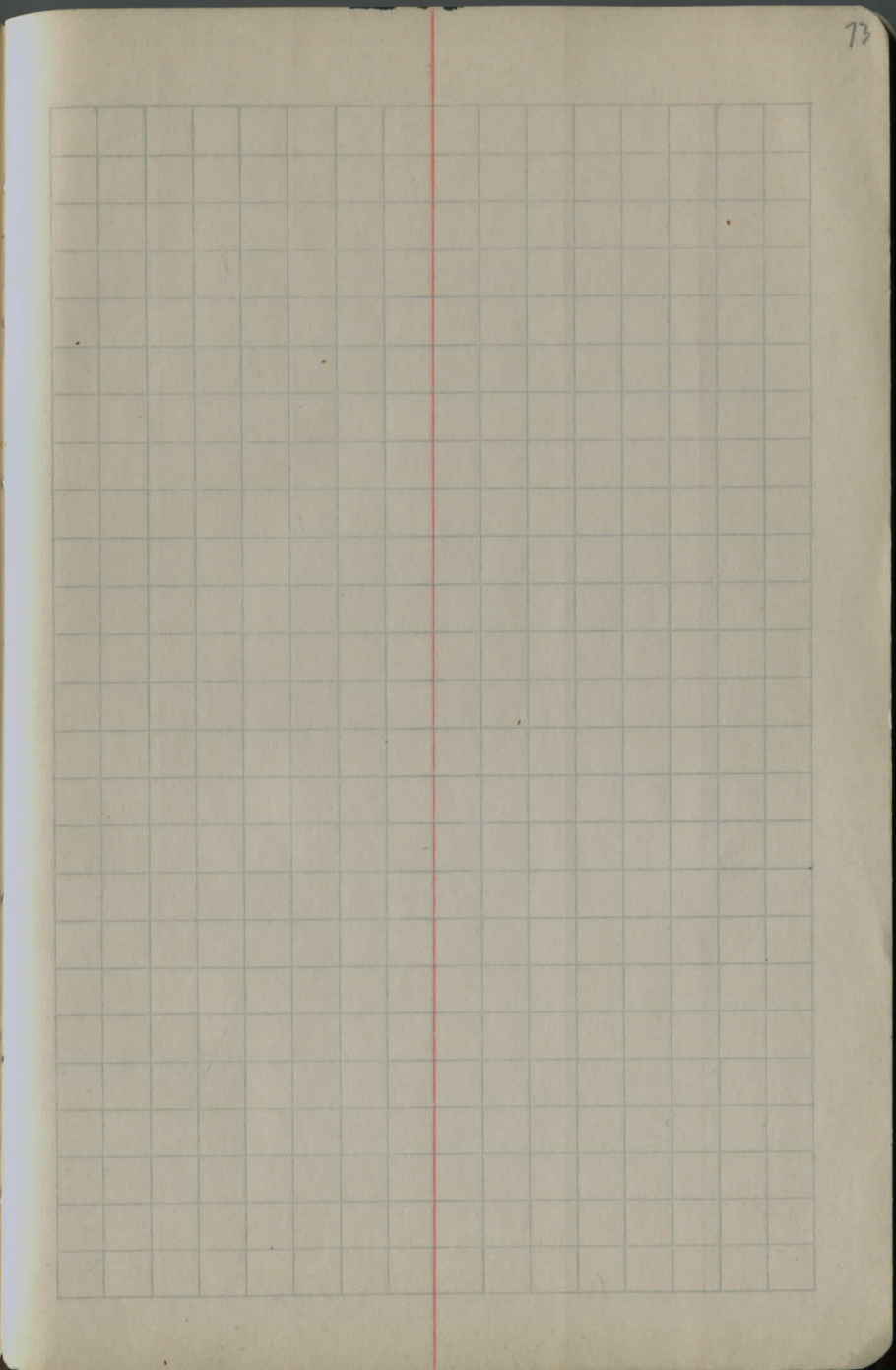
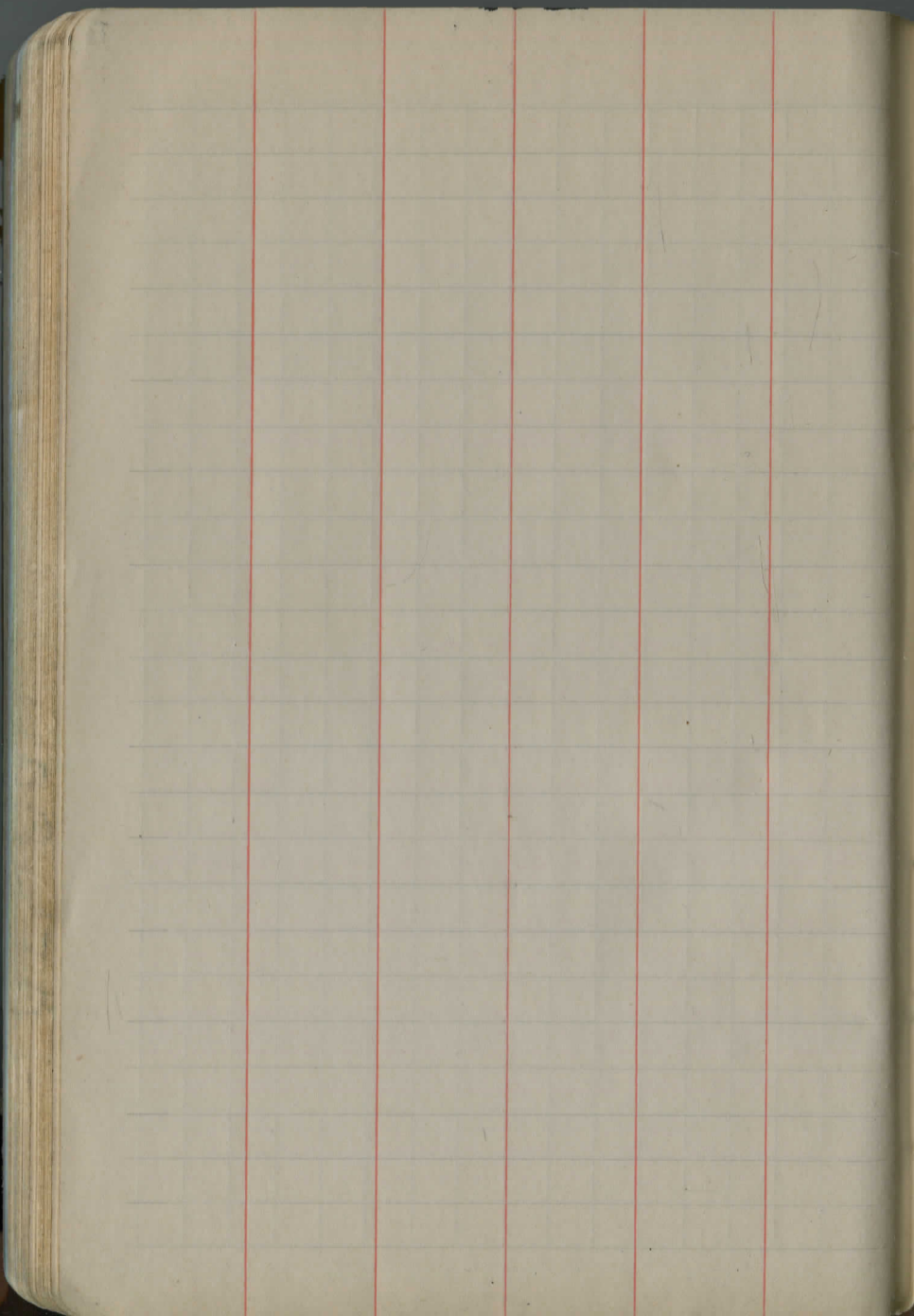


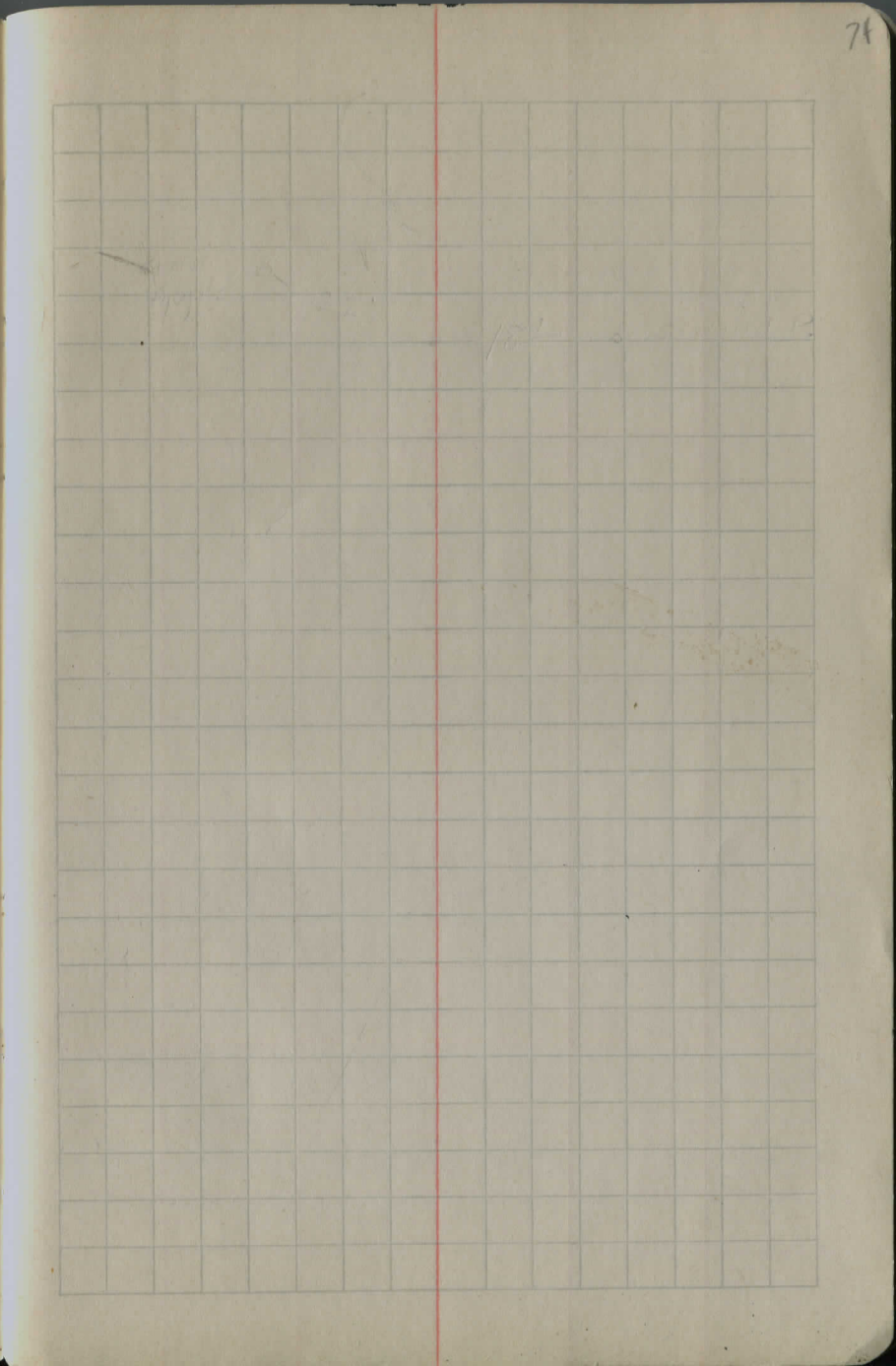
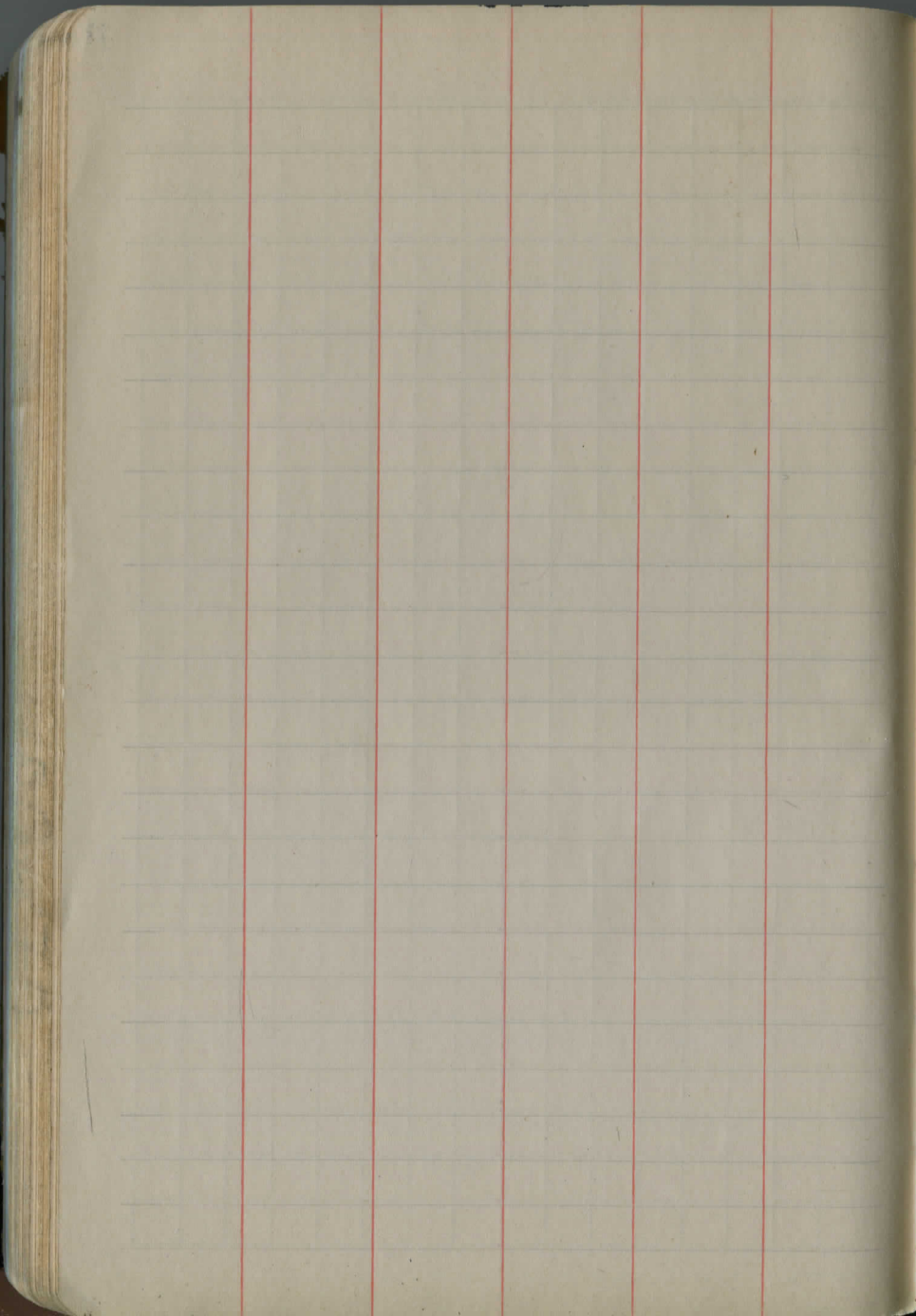


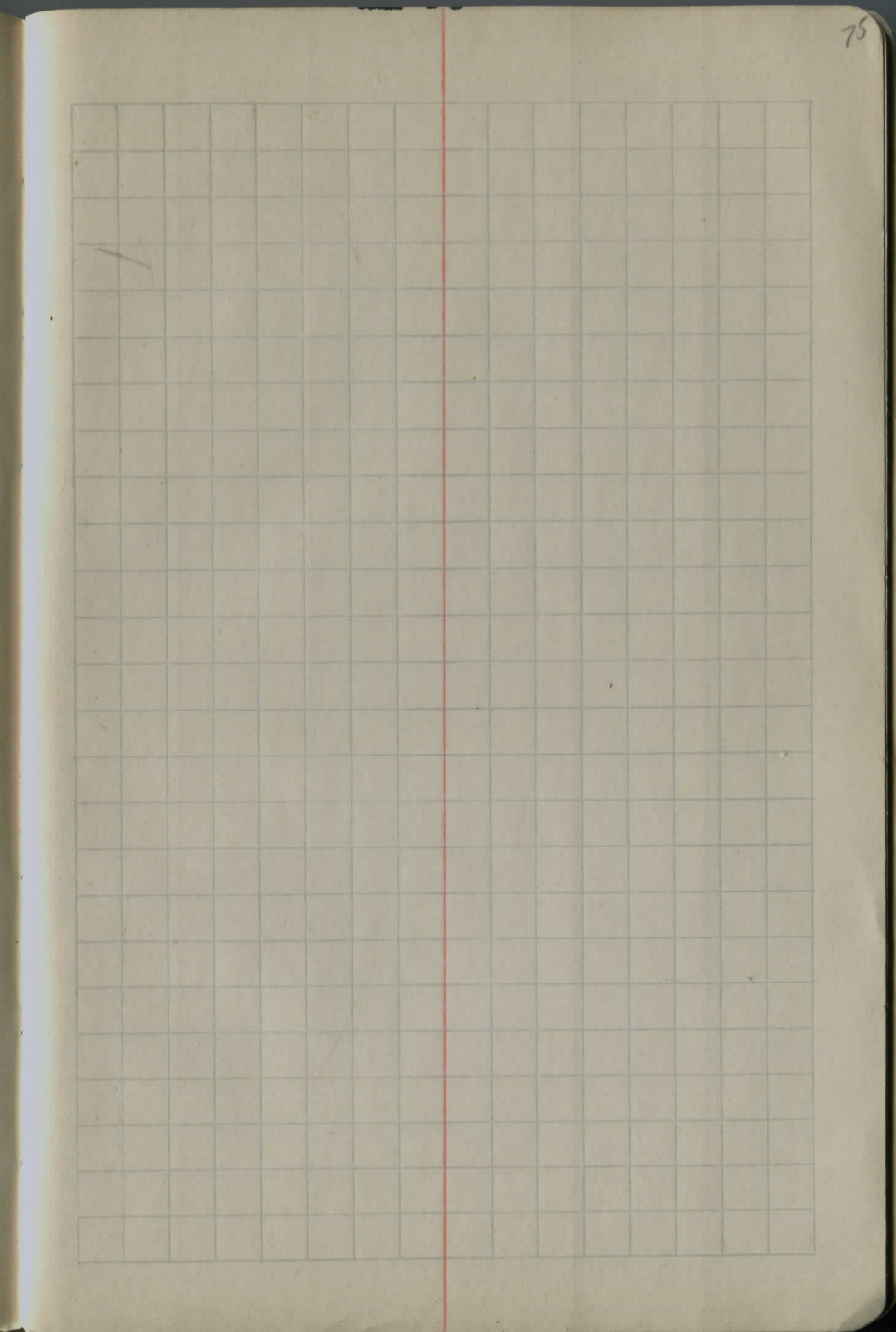
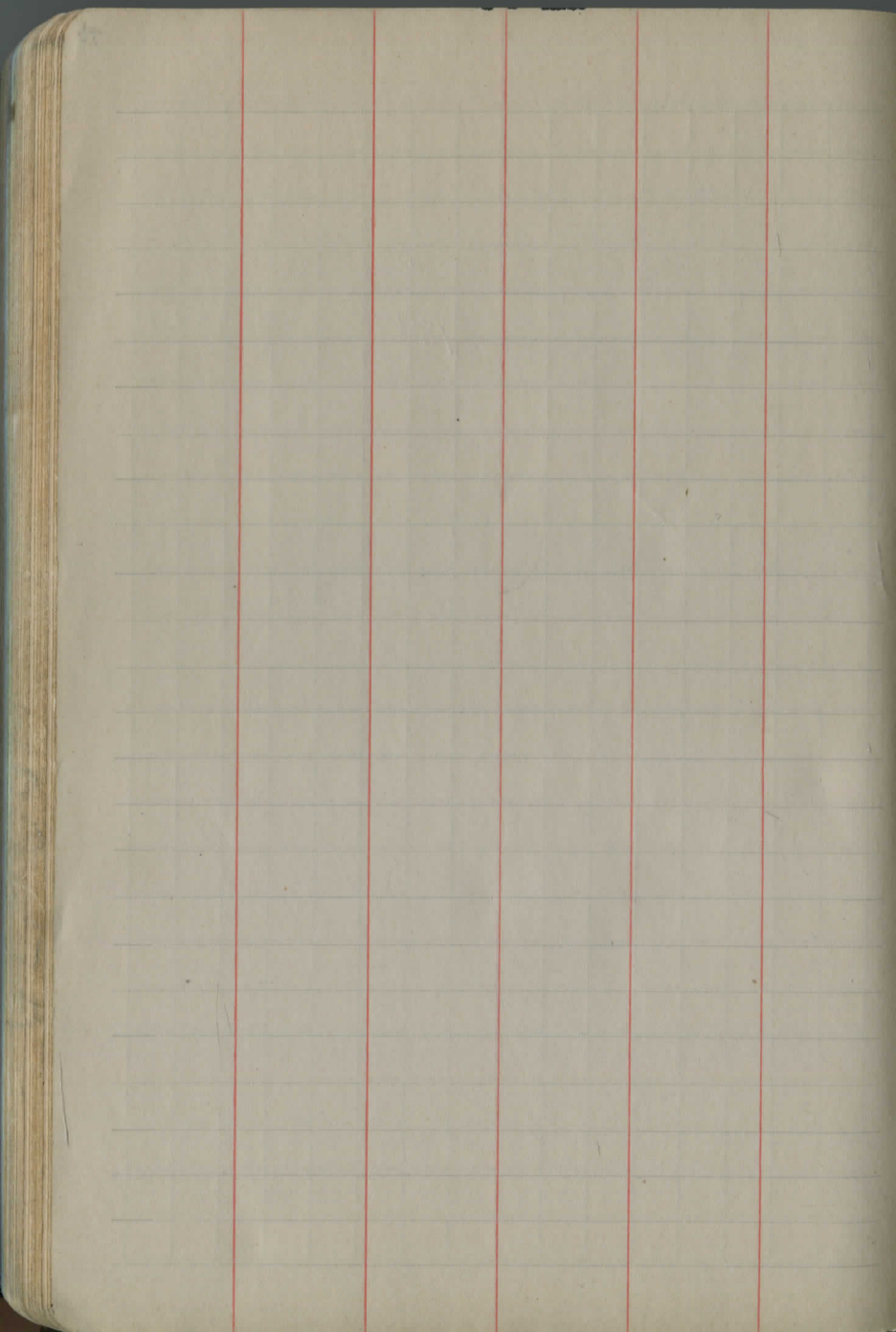


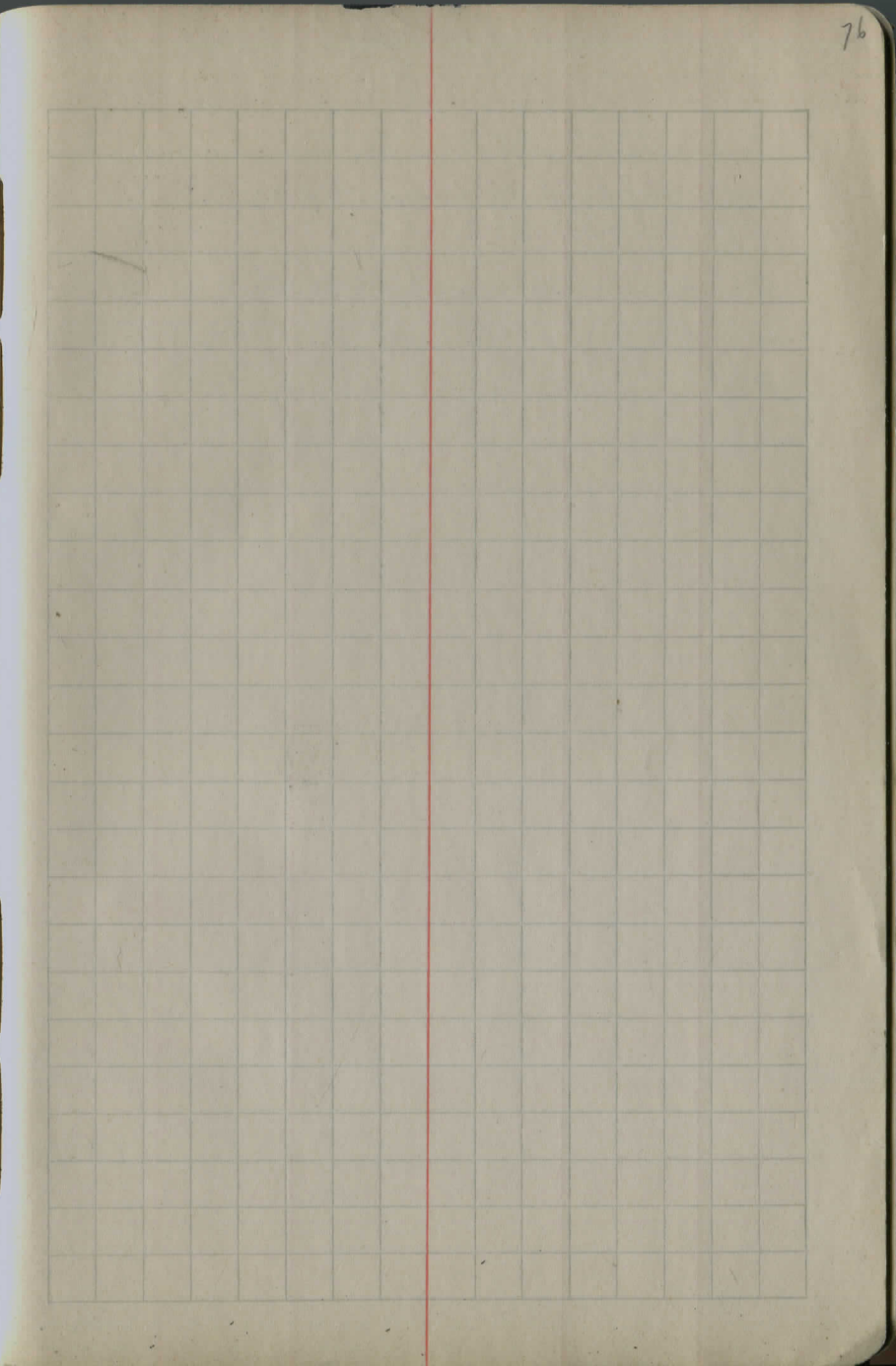
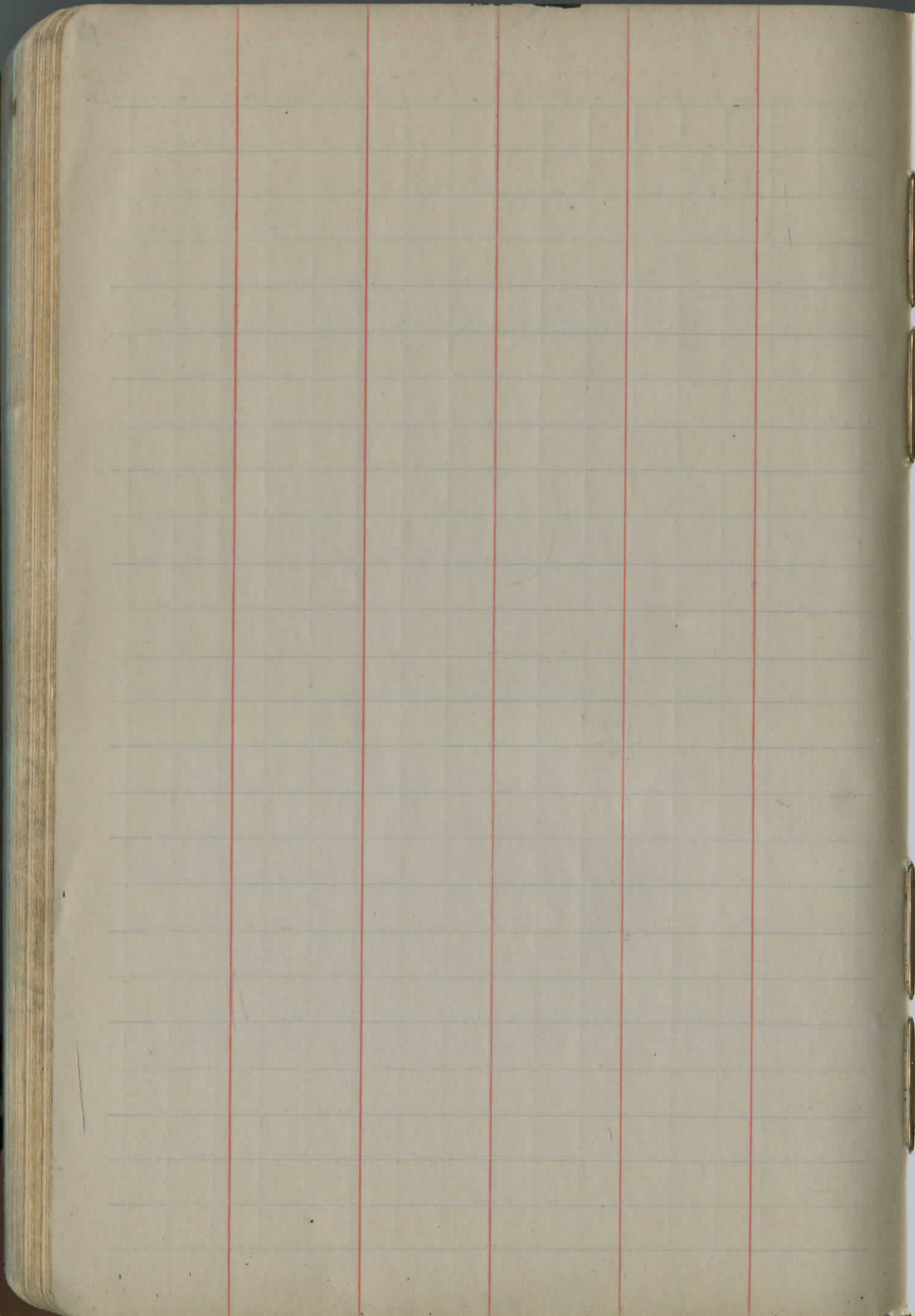


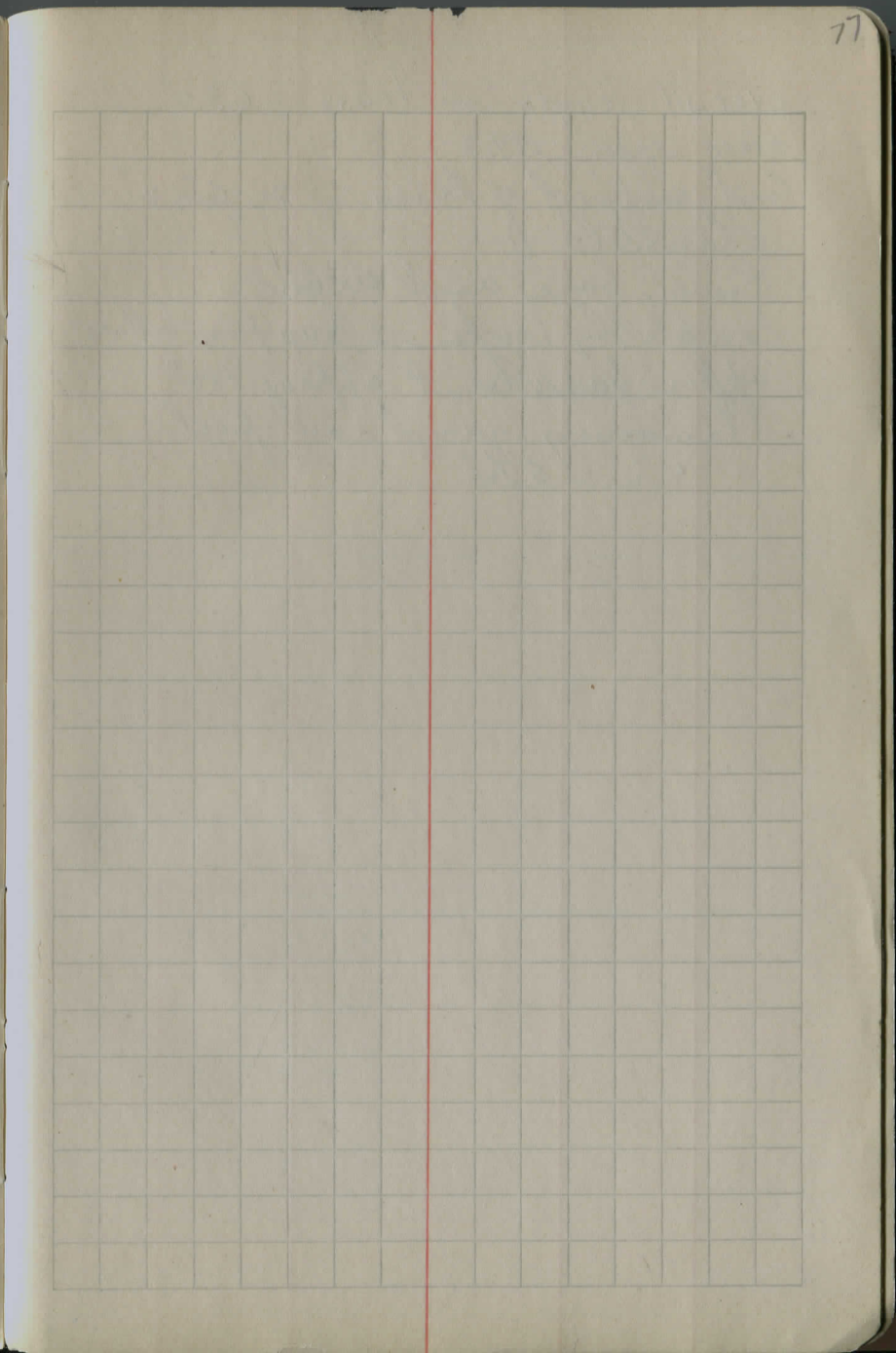
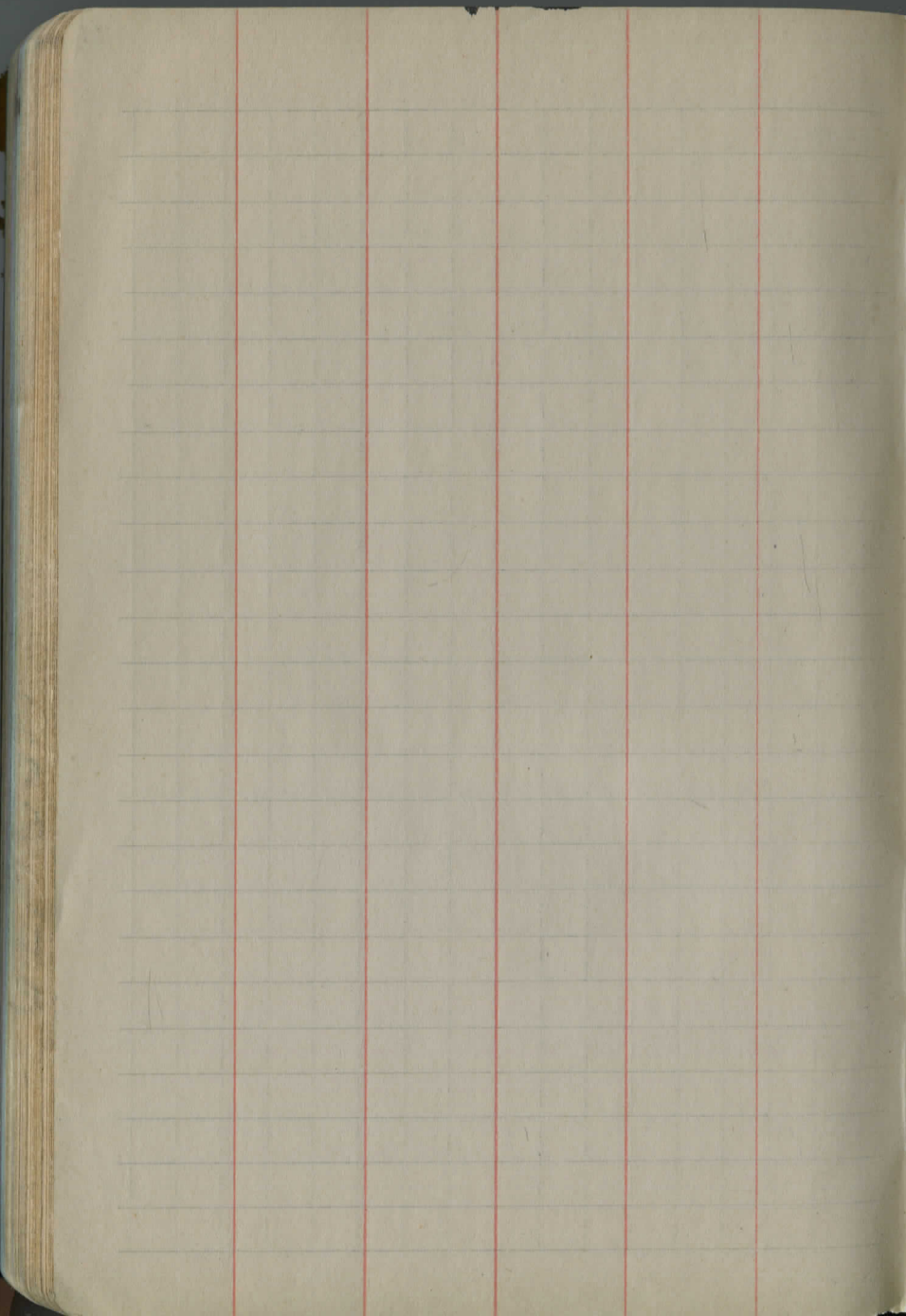












overall length of truss = 63.0'

clear span 59.3

end support of truss = 2 6"x2"  
channels

Lower base and web

completely broken  $\pm 1/3$  up from bottom

upper base bent approx  $180^\circ$

remaining guard rail post  
=  $4\frac{1}{2}" \times 5\frac{1}{2}"$

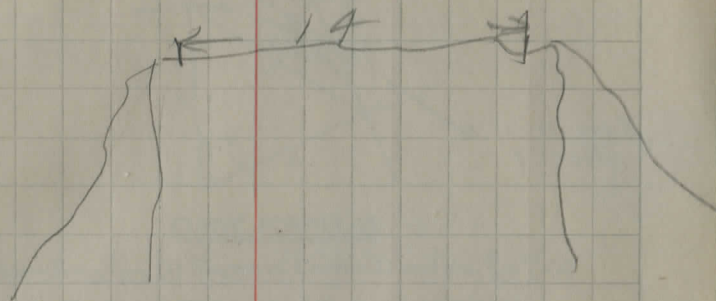
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Jalcott Hill 200 cyds.

Hill 350 " "

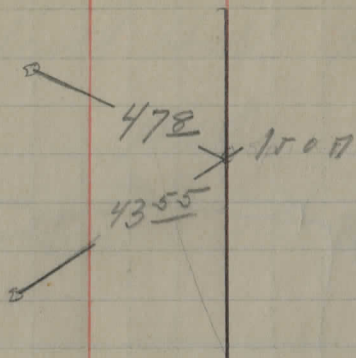
Bottom 300 cyds.

---

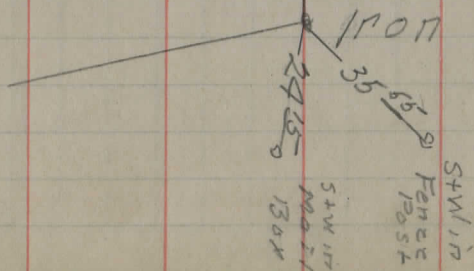


Ref North Point  
on Location for Hill

S+Win  
Cherry

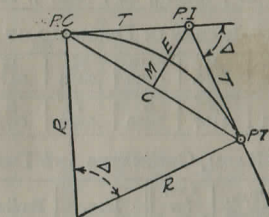


S+Win  
Cherry



# DIETZGEN'S RAILROAD CURVE AND REDUCTION TABLES

Copyright, 1914, by Eugene Dietzgen Co., New York City



## CURVE FORMULAS

Radius= $R = \frac{50}{\sin. \frac{\Delta}{2}}$  (1) Degree of Curve= $D$  and  $\sin. \frac{D}{2} = \frac{50}{R}$  (2)

Tangent= $T = R \tan \frac{\Delta}{2}$  (3) Length of Curve= $L = 100 \frac{\Delta}{D}$  (4)

Middle ordinate= $M = R(1 - \cos. \frac{\Delta}{2}) = R \text{vers} \frac{\Delta}{2}$  (5) (6)

External= $E = T \tan \frac{\Delta}{4}$  (7)  $= R \div \cos. \frac{\Delta}{2} - R$  (8)  $= R \text{exsec} \frac{\Delta}{2}$  (9)

Long Chord= $C = 2 R \sin. \frac{\Delta}{2}$  (10)  $\Delta =$  Central Angle

## EXPLANATION AND USE OF TABLES

**Stations.**—Given P. I.=Sta. 161+60.35 to find Sta. of P. C. and P. T.  $\Delta=62^{\circ} 10'$   $D=8^{\circ} 20'$ . From Table IV for  $1^{\circ}$  curve  $T=3454.1$  and  $\div 8\frac{1}{3}=414.49$  ft. From Table V correction=.36 or  $T=414.85$  ft. P. C.=Sta. P.I.— $T=157+45.50$ . Also from (4)  $L=746.00$  and P. T.=Sta. P. C. + $L=164+91.50$ .

**Offsets.**—Tangent offsets vary (approximately) directly with  $D$  and with square of the distance. Thus tangent offset for Sta. 158 on above curve is 2.16 ft. found as follows. From Table III tangent offset for 100 ft.=7.27 ft. Distance= $158 - \text{Sta. P. C.}=54.50$ , hence offset= $7.27 (54.50 \div 100)^2=2.16$  ft. Also square of any distance divided by twice the radius equals (approximately) the distance from tangent to curve. Thus  $(54.50)^2 \div (2 \times 688.26)=2.16$  ft.

**Deflections.**—Deflection angle= $\frac{1}{2} D$  for 100 ft.,  $\frac{1}{4} D$  for 50 ft., etc. For  $c$  ft.=(in minutes)  $.3 \times C \times D^{\circ}$  or=defl. for 1 ft. from Table III  $\times C$ . For Sta. 158 of above curve= $.3 \times 54.5 \times 8\frac{1}{3}=136.2'$  or  $2^{\circ} 16.2'$ , or= $2.50 \times 54.5=136.2'$  from Table III. For Sta. 159 deflection angle= $2^{\circ} 16.2' + 8^{\circ} 20' \div 2=6^{\circ} 26.2'$ , etc.

**Externals.**—May be found in similar manner to tangents. Thus  $E$  for curve above is 91.37. For from Table IV for  $1^{\circ}$  curve  $E=960.6$  for  $8^{\circ} 20' = 960.6 \div 8\frac{1}{3}=91.27$  and from Table V correction=.10 or  $E=91.37$  ft. Or suppose  $\Delta=32^{\circ}$  and  $E$  is measured and found to be 42 ft. What is  $D$ ? From Table IV  $E=230.9$  and  $\div 42=5.5$  or  $D=5^{\circ} 30'$ .

93.75  
 1.67  
 92.08

179-59-60  
 10-52-30  
 103° 07' - 30"

93  
 1112  
 188

179-60  
 87-29  
 92-31  
 16-52-30  
 109-23-30

93  
 9187.5  
 112.5  
 27.1  
 85.36

56+30.3  
 4974.2  
 656.1

10284  
 90.75  
 1209

360  
 177-08  
 537-08  
 179-02-40

63 63.7  
 56 30.3  
 732.8

DISTANCES FROM CENTER OF ROADWAY FOR  
 CROSS-SECTIONING.

PLEASE RETURN TO  
 GEORGE COUNTY ENGINEER  
 COURT HOUSE  
 CHARDON, O.  
 PHONE 250-X

H	0	.1	.2	.3	.4	.5	.6	.7	.8	.9	H
0	8.0	8.2	8.3	8.4	8.5	8.6	8.7	8.8	8.9	9.1	0
1	9.5	9.7	9.8	9.9	10.0	10.1	10.2	10.3	10.4	10.6	1
2	11.0	11.2	11.3	11.4	11.5	11.6	11.7	11.8	11.9	12.1	2
3	12.5	12.7	12.8	12.9	13.0	13.1	13.2	13.3	13.4	13.6	3
4	14.0	14.2	14.3	14.4	14.5	14.6	14.7	14.8	14.9	15.1	4
5	15.5	15.7	15.8	15.9	16.0	16.1	16.2	16.3	16.4	16.6	5
6	17.0	17.2	17.3	17.4	17.5	17.6	17.7	17.8	17.9	18.1	6
7	18.5	18.7	18.8	18.9	19.0	19.1	19.2	19.3	19.4	19.6	7
8	20.0	20.2	20.3	20.4	20.5	20.6	20.7	20.8	20.9	21.1	8
9	21.5	21.7	21.8	21.9	22.0	22.1	22.2	22.3	22.4	22.6	9
10	23.0	23.2	23.3	23.4	23.5	23.6	23.7	23.8	23.9	24.1	10
11	24.5	24.7	24.8	24.9	25.0	25.1	25.2	25.3	25.4	25.6	11
12	26.0	26.2	26.3	26.4	26.5	26.6	26.7	26.8	26.9	27.1	12
13	27.5	27.7	27.8	27.9	28.0	28.1	28.2	28.3	28.4	28.6	13
14	29.0	29.2	29.3	29.4	29.5	29.6	29.7	29.8	29.9	30.1	14
15	30.5	30.7	30.8	30.9	31.0	31.1	31.2	31.3	31.4	31.6	15
16	32.0	32.2	32.3	32.4	32.5	32.6	32.7	32.8	32.9	33.1	16
17	33.5	33.7	33.8	33.9	34.0	34.1	34.2	34.3	34.4	34.6	17
18	35.0	35.2	35.3	35.4	35.5	35.6	35.7	35.8	35.9	36.1	18
19	36.5	36.7	36.8	36.9	37.0	37.1	37.2	37.3	37.4	37.6	19
20	38.0	38.2	38.3	38.4	38.5	38.6	38.7	38.8	38.9	39.1	20
21	39.5	39.7	39.8	39.9	40.0	40.1	40.2	40.3	40.4	40.6	21
22	41.0	41.2	41.3	41.4	41.5	41.6	41.7	41.8	41.9	42.1	22
23	42.5	42.7	42.8	42.9	43.0	43.1	43.2	43.3	43.4	43.6	23
24	44.0	44.2	44.3	44.4	44.5	44.6	44.7	44.8	44.9	45.1	24
25	45.5	45.7	45.8	45.9	46.0	46.1	46.2	46.3	46.4	46.6	25
26	47.0	47.2	47.3	47.4	47.5	47.6	47.7	47.8	47.9	48.1	26
27	48.5	48.7	48.8	48.9	49.0	49.1	49.2	49.3	49.4	49.6	27
28	50.0	50.2	50.3	50.4	50.5	50.6	50.7	50.8	50.9	51.1	28
29	51.5	51.7	51.8	51.9	52.0	52.1	52.2	52.3	52.4	52.6	29
30	53.0	53.2	53.3	53.4	53.5	53.6	53.7	53.8	53.9	54.1	30
31	54.5	54.7	54.8	54.9	55.0	55.1	55.2	55.3	55.4	55.6	31
32	56.0	56.2	56.3	56.4	56.5	56.6	56.7	56.8	56.9	57.1	32
33	57.5	57.7	57.8	57.9	58.0	58.1	58.2	58.3	58.4	58.6	33
34	59.0	59.2	59.3	59.4	59.5	59.6	59.7	59.8	59.9	60.1	34
35	60.5	60.7	60.8	60.9	61.0	61.1	61.2	61.3	61.4	61.6	35
36	62.0	62.2	62.3	62.4	62.5	62.6	62.7	62.8	62.9	63.1	36
37	63.5	63.7	63.8	63.9	64.0	64.1	64.2	64.3	64.4	64.6	37
38	65.0	65.2	65.3	65.4	65.5	65.6	65.7	65.8	65.9	66.1	38
39	66.5	66.7	66.8	66.9	67.0	67.1	67.2	67.3	67.4	67.6	39
40	68.0	68.2	68.3	68.4	68.5	68.6	68.7	68.8	68.9	69.1	40

Example—If point is 22.6 ft. above grade, how far should it be from center line to be a slope stake point? Ans. from Table 41.9. For same slopes but other widths of roadbed correct above figures by one-half difference in width of roadbed; thus in example above for 20 ft. roadbed distance will be 41.9 + (20—16) ÷ 2 or 2 ft. added to 41.9 = 43.9. For slopes of 1 on 1 see inside of front cover.

